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1	Modeling of chloride spatial variability in a reinforced concrete wharf from on-
2	site measurements
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14 Abstract

Chloride ingress by diffusion is the major deterioration process of reinforced concrete (RC) structures exposed to the marine environment. These structures have significant lengths or surfaces exposed to the outside environment. Due to the material variability (different concrete batches, vibrations) and exposure variability, the material experiences a spatial variability of the deterioration process. This paper presents the geostatistical analysis of in-situ chloride profiles, leading to the assessment of the spatial variability (SV) of both the chloride ingress itself and the parameters of the widely used Fick diffusion law (the average surface chloride content, C_{sa} , and the average chloride diffusion coefficient, D_a). 37 chloride profiles measured on both sides of the same spandrel beam of an RC wharf are studied, as well as the associated estimates of C_{sa} and D_a . From an initial selection of random field models, the geostatistical analysis consists in the evaluation of model parameters using a procedure that tests both data and model assumptions on the fly (ergodicity, stationarity, random field modeling). Combined with the calculation of information criteria for each model, this procedure allows to provide relevant geostatistical models for chloride ingress, C_{sa} and D_a , which render SV as well as measurement error. It is noteworthy that the estimation error can be neglected when focusing on the SV for the range of chloride content studied in this paper. The SV of the chloride content seems to depend on the depth, with a large variability within the convection zone, and much less and more stable in the diffusion zone with a practical range of about 70 cm. This order of magnitude is consistent with the range of SV calculated for C_{sa} (50 to 73 cm).

- 32 **Keywords**: Chloride ingress, Reinforced concrete, Marine environment, Spatial variability,
- 33 Random field, Statistical analysis.

1. Introduction

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Chloride ingress is among the main causes of degradation of reinforced concrete (RC) structures where the deterioration is more perceptible in marine environment. That leads to major maintenance costs (Bastidas-Arteaga & Schoefs, 2015; Stewart & Val, 2003). These structures offer to the environment important lengths or surfaces. Due to the variabilities of both the material (different concrete batches, vibrations) and exposure, they experience a spatial variability of their deterioration process. Accounting for realistic spatial correlation of the different parameters in structural safety models may lead to more realistic estimates on structural reliability and maintenance planning, knowing that reliability assessment is affected by spatial correlation (Schoefs et al., 2022; Stewart, 2004; Stewart & Mullard, 2007). Moreover, quantifying this property allows optimizing inspection

- 44 meshing on a structure (Gomez-Cardenas et al., 2015; Oumouni & Schoefs, 2021b; Schoefs et al., 45 2017a).
- The variability of the degradation parameters and their measurements can be described at different scales:
 - the point scale, characterizing the repeatability of measurements;

- the local scale of the material, characterizing the heterogeneity of the material for a scale lower than the Representative Elementary Volumes (REV) associated with each of the quantities of interest (Hill, 1963);
 - higher scales (structural elements, structure), characterizing the degree of spatial
 heterogeneity of the material and/or environmental conditions (exposure, loading, ...) for
 scales above the REVs.

The restitution of point and local variabilities requires a statistical modeling, translating the hypothesis of independence, respectively on (i) the measurements of parameters between distinct measurements and for a same measurement point and (ii) the parameter values between similar structures for the same REV dimension. The spatial variability (taking into account local and larger scales) cannot be solely captured by a statistical modelling because of the hypothesis of the existence of a certain degree of heterogeneity, i.e. spatial correlation, between measurements or between parameter values associated with different measurement points or REV on the same structure. This can only be translated by geostatistical modeling. The major challenge of those models is to determine stochastic properties of the Random Fields (RF), namely their marginal distributions, stationarity and the spatial correlations.

Chloride ingress has been investigated deeply since the 70's and it has been shown how complex is the process physically and chemically (Glasser et al., 2008; Kaushik & Islam, 1995; Ragab et al., 2016; Safhi et al., 2019; Wegian, 2010) and the role the material complexity (porosity) is

- playing (Qu et al., 2021). For the moment, available models of transfer and knowledge don't allow to represent accurately the spatial variability of pollutant in concrete (Ravahatra et al., 2017). That is the main reason why direct or indirect assessment of chlorides in situ is the only way to measure, analyze and model the spatial variability of chloride ingress. Then, two kinds of quantities of interest can be considered:
- the total chloride content C at several depths;

- their mean diffusion parameters C_{sa} and D_a , estimated from profiles of chloride content.
- During the last decades sensors have experiences significant progresses (Biondi et al., 2021; Torres-Luque et al., 2014, 2017; Watanabe et al., 2021; Zaki et al., 2015) but there are few concepts that measure the spatial distribution of chloride ingress (Fares et al., 2018; Lecieux et al., 2015, 2019) and their implementation is rare (Priou et al., 2019) even if their interest in terms of the value of information they provide was shown (Schoefs et al., 2022) especially because they allow to filter the signal (Oumouni & Schoefs, 2021a). Moreover, these sensors have to be installed at the building stage and are not suitable for existing structures.
- That is the main reason why spatial variability is assessed through Non-Destructive Testing (NDT) tools or Semi-Destructive Techniques (SDT). On-site indirect assessment of chlorides through NDT-techniques is affected by errors of assessment and sensitivity to other parameters (humidity, temperature) which makes it difficult to assess the spatial variability of chlorides (Bonnet et al., 2017; Bourreau et al., 2019; Schoefs et al., 2023). This error affects the optimization of measurements for assessing spatial variability (Oumouni & Schoefs, 2021b). That is why, the rare studies are based on the semi destructive techniques (SDT) i.e. direct weighting of chlorides from cores (O'Connor & Kenshel, 2013; Othmen et al., 2018) reviewed 10 published studies from 2004 to 2016 with only 3 concerning more than 30 measurements and in all cases the distance between measurement was too large in view to assess the spatial correlation. Two main quantities of interest are investigated: chloride content at a given depth and parameters of a model of chloride ingress after post-processing

of chloride profiles.

Because of the complexity and cost for making a large amount of cores in a beam, only a few amounts of data is available for each and the distance between cores is too large: 5 cores with a distance of about 1 m in the study of (O'Connor & Kenshel, 2013). That leads to a significant numerical error of assessment of the spatial correlation (Schoefs et al., 2017b). Moreover, the underlying assumptions for RF modelling that are stationarity and ergodicity are not checked. Finally, the error of measurement is usually not considered when identifying correlations. That is a drawback because it is well known that SDT measurements for existing structures (Schoefs et al., 2023) and even for concrete in laboratory (Bonnet et al., 2017; Hunkeler et al., 2000) are affected by a significant error. To overcome these limits, this paper relies on two pillars: first, a unique database and second, a geostatistical analysis that allows to provide geostatistical parameters (namely spatial correlation assessments as well as measurement error) by using a recent method of RF identification (SCAP1D, Clerc et al. 2019), that accounts for uncertainty of measurements, checks ergodicity and accounts for the non-stationarity of the RF.

Concerning the first pillar, marine structures can be exposed in various environments: atmospheric, spatter or splash, tidal and underwater (BS 6349-1:2000, 2000) which were defined according to the tide range (Bourreau et al., 2020). In this work, 30 cores carried out every 30 cm on the same 28-year beam (Othmen et al., 2018) are analyzed. The beam is located in the worst environment that is splash zone (Angst et al., 2009), and the cores are through its width giving access to profiles from each side i.e., exposure conditions (exposed, sheltered).

Concerning the second pillar, two objectives are targeted: the assessment of spatial variability of the total chloride content C, depending on depth and exposure on one hand and the assessment of the spatial variability of the mean diffusion parameters C_{sa} and D_a on the second one. First, taking benefit of the availability of C at several depth, the objective is to provide estimates of its geostatistical parameters and to answer the following questions: (i) Is the spatial variability (SV) of C a function of

depth? (ii) Is the SV of C a function of the exposure conditions? Second, knowing the age of the structure (28-year), geostatistical parameters of mean diffusion parameters are estimated to answer the following question: Is the SV of C_{sa} and D_a a function of the exposure conditions? The latter question arises because C_{sa} and D_a are directly dependent on the chloride profiles, which vary markedly between the exposed and sheltered faces.

Section 2 introduces the geostatistical formalism in a theoretical way before presenting a state of the art of RF modeling, simulation methods and spatial variability estimation methods. An attention is paid to the underlying assumptions of the estimation methods and to the consequences of their neglect on the estimated spatial variability, especially in the frequent case of non-stationary measurements due to spatially variable exposure conditions. Section 3 presents the structure and the available data. Sections 4 and 5 give the results at two levels: total chloride contents, and mean diffusion parameters of total chlorides. Section 6 concludes the paper with some extension to other structures or other exposures.

2. Geostatistical methods for spatial variability assessment of concrete structures

2.1. Modelling of spatial variability

- 133 2.1.1. Random Field modeling
- Practical modeling of spatial variability implies modeling spatially variable quantities of interest (QI)
- as Second-Order Intrinsic Stationary Random Fields (RF) (Chilès & Delfiner, 2012). Following the
- geostatistical formalism, such an RF Z is defined on a domain D^n of \mathbb{R}^n and on a probability space
- 137 (Ω, A, P) such that:

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- 138 $\forall x \in D^n$, $Z(x,\cdot)=Z_x$ is a multivariate random variable of $\omega \in \Omega$ with a joint probability density
- function (PDF) $f_Z(m, \Sigma)$, where **m** and Σ are the mean vector and the covariance matrix of Z_x ,
- respectively;

- ∀x ∈ Dⁿ and ∀ω ∈ Ω, Z(x, ω) = Z_x^ω is a realization of Z georeferenced by the vector x, also
 called a trajectory (in practice, a set of measurements from a unique structure and at a unique time).
- $\forall (h,x) \in D^2$, $\mathbb{E}[Z_{x+h} Z_x] = 0$ and $Var(Z_{x+h} Z_x) = 2\gamma(h)$ where $\gamma(\cdot)$ is the semivariogram function of Z and h is any distance-lag (distance between two points of D).
- This definition reflects both the variability of the input parameter between structures and its spatial variability within the same structure, which are embedded in the *covariance matrix* Σ and the semi-variogram.
- 149 In this paper, data are modeled as (i) unidimensional (ii) noisy (iii) trend-stationary (iv) Gaussian 150 Random Field (GRF). Indeed, (i) the data are unidimensional along the sides of the beam; (ii) if the 151 spacing between measurements is greater than the REV, the RF model must be able to reproduce the 152 point and local variabilities, which have no correlation structure and can be modeled by white noise; 153 (iii) materials and environmental conditions may vary along the sides of the beam. It is then very likely that the mean of the quantities of interest will vary with space; (iv) GRFs are very convenient 154 155 because they allow modeling most natural variabilities (following normal or lognormal marginal 156 laws) with only a model for the mean and the covariance (Chilès & Delfiner, 2012).
- 157 Mathematically, $\forall x \in D^n$ we thus consider Z such that

$$Z_x = \boldsymbol{m}_x + W_x + Y_x \tag{1}$$

with m_x a deterministic trend, $W_x \sim N(\mathbf{0}, t \times \mathbf{1}_n)$ a white noise of variance t; $Y_x \sim N(\mathbf{0}, \Sigma_Y)$ a centered GRF, i.e Y_x a multivariate normal variable such that

$$\forall (h, x) \in D^2, \Sigma_{Y_{x+h, x}} \equiv Cov(Y_{x+h}, Y_x) \equiv C_Y(h) \equiv s \times \rho_Y(h) \quad (2)$$

where $C_Y(\cdot)$ is the covariance function of Y, s is its variance, $\rho_Y(\cdot)$ is its autocorrelation function, and h is any distance-lag between the data (equivalences due to the Second-Order Stationarity of the GRFs).

- 163 This formalism is illustrated in Figure 1 and Figure 2, where trajectories, correlation plots,
- autocorrelation functions, and semivariograms are plotted for *Y* and *Z* respectively, defined such that:

165 1)
$$Y_x \sim N(\mathbf{0}, \Sigma_Y)$$
 with $\Sigma_{Y_{x+h,x}} = s \times \exp(-1.73 \|h\|^2 / r_{95})$, $\rho_Y(h) = \frac{\Sigma_{Y_{x+h,x}}}{s}$, $\gamma_Y(h) = s - \frac{\Gamma_{Y_{x+h,x}}}{s}$

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$$\Sigma_{Y_{x+h,x}}$$
, $s = 10$ and $r_{95} = 20$;

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2)
$$Z = W + Y$$
 with $W_x \sim N(\mathbf{0}, t \times \mathbf{1}_n)$, $\Sigma_{Z_{x+h,x}} = t \times \delta_{x+h,x} + \Sigma_{Y_{x+h,x}}$, $\rho_z(h) = \frac{\Sigma_{Z_{x+h,x}}}{s+t}$,

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$$\gamma_Z(h) = s + t - \Sigma_{Z_{x+h,x}}$$
, and $t = 5$ (δ_{ij} is the Kronecker delta).

169 2.1.2. Autocorrelation models and parameters

3. Embedded Tables

Table 5 and Figure 1 present and illustrate the exponential and Gaussian autocorrelation functions, widely used in the literature. According to Equation (2), such functions are positive definite, so that they asymptotically decay as a function of the distance-lag h. The evolution of this decay is mainly determined by the value of their scale parameter a > 0, which parametrizes the extent of the spatial correlation, i.e. the distance threshold below which the QI are strongly correlated.

The distance at which events are weakly correlated is referred to as the inspection distance threshold with a spatial correlation threshold definition of 30% in (Schoefs et al., 2016). Several notations and parameters coexist in the literature to describe this threshold, sometimes with some confusion. In the case of isotropic fields, the following three notations are commonly used: (i) Cressie (1993, p. 118) proposed the fluctuation range θ , which is defined as $\theta = \int_{-\infty}^{+\infty} \rho(h) dh$; (ii) (Cressie, 1993, p. 88) proposed the practical range r_{95} , which is defined as $\rho(r_{95}) = 5\%$ (see Figure 1 and Figure 2); (iii) many authors proposed the correlation length l_c , which is not strictly defined and alternatively taken to be equal to a, θ , r_{95} or some other particular range. In order to avoid any confusion and to facilitate comparison and interpretation, we give in

4. Embedded Tables

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Table 5 the expression of θ and r_{95} as a function of a for the exponential and Gaussian models and the corresponding autocorrelation values ρ . Note that in Figure 1 Y is obtained from a Gaussian autocorrelation function.

4.1. Identification of spatial variability from measurements

- 193 4.1.1. Empirical identification
- 194 RF models behind a trajectory can be identified in the case of its *ergodicity*, which can be seen as a
- mixture of stationarity and asymptotic independence. In this case, the spatial variability can be
- assessed from empirical representations of the semivariogram, the covariance function, or the
- 197 autocorrelation function, called the empirical semivariogram, covariogram and autocorrelogram,
- 198 respectively.
- The Semivariogram, denoted $\hat{\gamma}(h)$, is used because its estimation does not require complementary
- ones (such as mean or variance), and it allows to visually identify (i) the Second-Order Stationarity
- property, (ii) the ergodicity property, and (iii) the presence and magnitude of local variabilities.
- Indeed, (i) $\hat{\gamma}(h)$ asymptotically tends to the variance for Second-Order Stationary RF, while otherwise
- whereas it increases with h; (ii) ergodicity is verified when $\hat{\gamma}(h)$ reaches an asymptote in the limit of
- half of the domain D (Cressie, 1993, p. 57); (iii) white noise is easily identified by a nugget effect,
- 205 i.e. a non-zero value for h = 0 (see Figure 2).
- 206 A common estimator of $\hat{\gamma}(h)$ is defined by Equation (3) for a given trajectory Z_x^{ω} , where S_h is the set
- of pairs (x_p, x_q) of D^2 that are away from h modulo a given tolerance, and N_h is the cardinal of S_h .

$$\widehat{\gamma}(h) = \frac{1}{2N_h} \sum_{(x_p, x_q) \in S_h} \left(Z_{x_p}^{\omega} - Z_{x_q}^{\omega} \right)^2 \tag{3}$$

- We point out that when h becomes close to the dimension of D, statistical inference is no longer valid because the number of point pairs away from h is very small. Therefore, geostatisticians
- 210 recommend plotting this graph only for distances h such that $N_h \geq 30$ (Cressie, 1993). In practice,

- 211 in civil engineering, several authors recommend limiting to values of h less than half of the size of
- the domain (Arnaud & Emery, 2000, Schoefs et al., 2016).
- 213 *4.1.2. Estimates of model parameters*
- Once an RF model is selected from empirical identification, its parameters (collected in a vector **p**)
- 215 must be estimated. Several methods are available:
- Maximum Likelihood Estimation (MLE), which allows to estimate **p** from raw data, given an RF
- marginal distribution model, and as the following properties: (i) asymptotically unbiased, (ii)
- consistent, (iii) equivariant, (iv) asymptotically efficient, and (v) asymptotically normal
- 219 (Wasserman, 2004, para. 9.4);
- Least-Squares Estimate (LSE), which allows estimating **p** from semivariogram, covariogram or
- 221 autocorrelogram given models of them;
- Weighted LSE (WLSE), similar to LSE, where each of the residuals is weighted by its variance.
- 223 This allows to increase the weight of the residuals computed from many data (low distance-lag h
- values, for which N_h is larger).
- Usually, LSE and WLSE fit the covariograms well and the use of WLSE implies a better description
- of them for small distances, associated with more data¹. However, covariograms are not necessarily
- 227 representative of the effective covariance parameters of the field, which can lead to significant relative
- errors in their estimation. Because it considers only raw data, MLE provides more accurate estimates,
- with smaller relative errors. The trade-off is computational time, as MLE is more time consuming
- 230 than WLSE and LSE, up to 10 times faster. Therefore, and because it is a priori complex to have an
- 231 initial idea of the scale parameter, a two-phase estimation method of Second-Order Stationary RF
- parameters is preferred:

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¹ In this case, $\gamma(h) = 1 - C(h)$, so estimates on the covariogram or the semivariogram are equivalent.

- 1. WLSE pre-estimation of the parameters with the following initial values: (i) for *s*, the empirical variance; (ii) for *a*, a value such that the fluctuation scale is less than a quarter of the maximum
- lag-distance (so as to respect the ergodic assumption);
- 236 2. MLE estimation of the parameters using the Nelder-Mead simplex minimization algorithm with
- 237 the WLSE estimates as initial values.
- 238 This approach allows to obtain accurate estimates while minimizing the cost of the MLE due to the
- definition of initial values close to the real values of the parameters (Clerc et al., 2019, para. 4.4).
- 240 4.1.3. Choice of best RF model
- 241 Although the plot of the empirical semi-variogram allows the selection of an RF model, its estimator
- 242 nature leads to an imperfect representation of the SV. This is all the more true when the number of
- 243 measurements is small. Therefore, when there is insufficient scientific evidence to support a specific
- 244 RF model for a QI, which is the case for chloride content and diffusion parameters (Gomez-Cardenas
- et al., 2015), we can rely on data-based methods for model selection.
- In the following, the Akaike Information Criterion (AIC) is preferred because it is much more robust
- than the R^2 coefficient of determination (Burnham & Anderson, 2010). The AIC allows to compare,
- with the parsimony criterion, the relevance of any RF model M_Z with K parameters $p_{1,...,K}$ to describe
- the hidden RF Z at the origin of a set of n data Z_x^{ω} (e.g., a trajectory). This criterion assumes that the
- best model is the one that provides the most accurate description with the fewest possible parameters
- 251 (Burnham & Anderson, 2010, para. 1.4).
- In particular, when n/K < 40 and (which is the case below) we use the corrected information
- criterion (AICC). When the p estimate \hat{p} is computed with MLE, it writes:

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$$AIC_c = \underbrace{-2\ln(L(\hat{p}|Z_x^{\omega})) + 2K}_{AIC} + \underbrace{\frac{2K(K+1)}{n-K-1}}_{\text{correction}}$$

- where $L(\hat{p}|Z_x^{\omega})$ is the likelihood of \hat{p} with respect to the data. The best model among a set of
- considered models is then the one that minimizes the AICC, i.e. the one that has both a high goodness
- of fit (high likelihood) and a limited complexity (limited number of parameters).

In practice, the easiest way to compare the relevance of models based on their AICC is via the Evidence Ratio (ER) (Burnham & Anderson, 2010, para. 2.10). This is defined such that, for any model $M_{Z,i}$ among N_m models,

$$ER_{i} = \exp\left(\frac{1}{2}\left[AICc_{i} - \min_{\substack{j=1,\dots,N_{m} \text{best } a \ priori \ \text{model}}}(AICc_{j})\right]\right)$$

The ER is therefore 1 for the best *a priori* model and grows exponentially as the models become less fit. From an expert perspective, an ER of 2 for a given model does not justify preferring the best a priori model over it, whereas an ER of 18 clearly favors the best a priori model (Burnham & Anderson, 2010, para. 2.10).

4.1.4. SCAP1D: a robust estimation procedure

The methods presented assume an adequate RF model to avoid estimation errors: without modeling precautions, they are only valid under the condition that the data can be described by an SSO and ergodic RF. However, since classical regression techniques coupled with generic autocorrelation models can easily approximate autocorrelograms and covariograms, the estimation of RF model parameters is typically performed by LSE on raw data without prior verification of these two hypotheses (O'Connor & Kenshel, 2013; Ravahatra et al., 2017; Schoefs et al., 2016). Such a procedure can then lead to significant errors, in especially in the estimation of the scale parameter α , and especially in cases of non-stationarity in the mean and/or significant measurement noise. This point is demonstrated in a case study by (Clerc, 2021, p. 143): from a given realization of a standard GRF, adding a noise and/or a mean trend and performing an LSE of α without prior verification of the SSO and ergodic hypotheses led to estimation errors of up to 994%.

To avoid these errors, we use here the SCAP-1D procedure² (Clerc et al., 2019). Based on previously

presented tools, it allows to estimate, from a single trajectory with few measurement points, the

² Spatial Correlation Assessment Procedure for Unidimensional Data

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- parameters of a unidimensional noisy trend-stationary RF with piecewise polynomial mean, constant
- variance and constant autocorrelation model.
- The concept of SCAP-1D is to ensure the reliability of the parameters' estimates by testing and
- validating the following hypotheses:
- H1: validity of the RF mean model and its parameter estimates, i.e. Second-Order Stationarity of
- 285 the centered RF;
- H2: validity of both the joint PDF and autocorrelation function models of the RF and of the
- estimation of their parameters; i.e., normality of the standard source GRF, obtained by
- isoprobabilistic transformation of the RF from estimates considering the *a priori* model;
- H3: ergodicity of the trajectory; i.e., non-correlation of the data at infinity or determination of all
- 290 geostatistical parameters from this trajectory.
- The SCAP-1D flowchart is shown in Figure 3. It is divided into four consecutive steps, which can be
- repeated if the estimates do not validate the hypotheses on the RF model:
- Step I) Choice of SCAP-1D parameters, namely piecewise polynomial mean model (regression
- degree, number of changepoints), RF joint PDF and autocorrelation function. We recommend to
- 295 plot the trajectory, its histogram and its experimental semivariogram to facilitate this step.
- <u>Step II) Estimation of the mean and geostatistical parameters</u>: changepoints in the mean and their
- significance are determined using the PELT (Pruned Exact Linear Time) optimal partitioning
- algorithm of (Killick et al., 2012). Estimation of the RF parameters is then performed using the
- approach presented in $4.1.2^3$.
- Step III) Checking of the underlying mathematical hypotheses: the plausibility of the estimated
- model is verified by checking ergodicity and performing stationarity tests (DF-test (Dickey &
- Fuller, 1981), KPSS-test (Kwiatkowski et al., 1992)) and normality tests (χ2, KS-test
- 303 (Kolmogorov-Smirnov)) on the standardized trajectory, based on the Step II estimates.

³ or by WLSE on the experimental semivariogram in the case of non-explicit density models.

- <u>Step IV) Analysis</u>: based on the Step III tests, the model selected in Step I and the estimates made in Step II are either accepted (algorithm stops) or rejected (modification of Step I parameters is required).
- In Step III, the standardization of the trajectory is done by the Cholesky transformation, taking into account either:
- the empirical covariance matrix: in this case, we only test H1, since the tests results depend only on the data and the estimates of the mean. The tests are then called Σ_{exp} -tests;
- or the modeled covariance matrix: in this case, we test H1 to H3 together, since the tests results depend on the total estimates. The tests are then called Σ_{mod} -tests.
- 313 *4.1.5. Interpretability criterion*
- In Step IV of SCAP-1D, the estimates must be analyzed for their physical consistency. In particular, an interpretability criterion can be defined for the estimated practical range. The point is to ensure, with respect to the distance Δx between measurements, that the variability thus described is indeed a spatial variability and not a local or point variability. This is illustrated in Figure 4, where the Gaussian autocorrelogram is plotted on a measurement grid of step $\Delta x = 5$ for three practical range values $(r_{95} < 2\Delta x, r_{95} \simeq \Delta x, \text{ and } r_{95} > 2\Delta x)$:
- when $r_{95} \ge 2\Delta x$ (curves c3 and c2): the variability described is spatial variability, since there is at least one significant intermediate spatial correlation value between the total correlation of a point with itself and the minimum 5% correlation associated with the practical range;
- when $r_{95} < 2\Delta x$ (curve c1): the variability described may be local or point variability since there is no significant intermediate spatial correlation value to account for the spatial variability.

Therefore, care will also be taken to ensure that the criterion $r_{95} \ge 2\Delta x$ is met when applying SCAP-326 $1D^4$.

5. Presentation of the structure and the measurements

5.1. Presentation of the wharf, the beam and its material

The reinforced concrete beam inspected within the framework of the APOS research project is a T hyperstatic beam called "J-beam", whose dimensions and reinforcement are shown in Figure 5. It is located on lane 51 of the gangway 5 of the Montoir-de-Bretagne coal terminal (Loire-Atlantique department, France) with GPS coordinates: 47.32°N, -2.17°E. This terminal, built between 1981 and 1983 on the banks of the Loire and less than 7 km from the Atlantic Ocean, is 234.4 m long and 20.7 m wide and is located at +8.40 m NGF (Figure 6). This proximity to the ocean implies the existence of a tidal phenomenon with an amplitude of 5.80 m and a salinity of the water corresponding to maritime conditions at high tide (about 30 g Cl⁻/L). Thus, at the highest tidal coefficients, the lower face of beam J, located 5.80 m above the level of the Loire, is in contact with the water for about 2 days per month. Finally, one of the peculiarities of the J-beam is its location at the edge of the quay. It has a side exposed to the wetting-drying cycles, marked Ext, and a sheltered side, marked Int (see Figure 6).

Data from the construction archives indicate that the concrete used to build the structure is a Portland cement concrete CEMI 42.5N dosed at 350 kg/m3. The mass fraction of m_{cl} clinker is therefore higher than 90% and that of secondary constituents such as gypsum does not exceed 5%. The aggregates used are sand 0/6, and gravel 5/10 and 10/25. Concrete water porosity ε , measured on the central part of 5 specimens used for chloride titration, was 13.7% on average, with minimum and

⁴ This criterion is constructed similarly to that of (Der Kiureghian & Ke, 1987) on the ability of a discretization grid to reproduce the spatial variability properties of an RF.

maximum values of 11.3% and 15.9%, respectively. The average compressive strength f_{cm} , measured on 4 specimens of 5 cm diameter and 10 cm length, was 43.5 MPa, with minimum and maximum values of 38.5 MPa and 48.9 MPa, respectively.

5.2. Total chloride content measurements at 28 years

351 5.2.1. Available data set

Chloride content measurements were performed on 30 through cylindrical cores extracted on the J-beam at the same height of 30 cm and with a lateral spacing of 30 cm (see Figure 7) (Othmen et al., 2018). The extraction was performed in 2011-2012, after 28 years of exposure. From initial dimensions of 50 mm in diameter and 400 mm in length, each core was divided into 5 slices (identified in Figure 7). Slices 1, 2, 4 and 5 were used to determine the chloride profiles according to the procedure recommended by the RILEM TC 178-TMC (Vennesland et al., 2013). Slices 1 and 2 are related to the outer side (collected in Ext specimens) and slices 4 and 5 are related to the inner side (collected in Int specimens). While deep slices 2 and 4 were 40 mm thick with measurements in 10 mm increments, slices 1 and 2 were 45 mm thick with measurements in 5 mm increments to capture the value at the surface. Slice 3 was used for further investigations such as porosity, compressive strength and estimation of the initial chloride content. Out of a total of 30 samples per side, 16 were usable on the Ext Face (53%), and 21 on the Int Face (70%). Furthermore, of the 555 total chloride contents of the 30 samples considered, only 460 were usable (83%, or 51% of the total originally planned measurements).

Figure 9 hereafter then represents the remaining set of total chloride measurements at 28 years finally considered (marked *C* in the following). The evolution of the empirical mean and standard deviation

by depth and by face is shown in Figure 8.

370 5.2.2. Constitution of trajectories

In the following, the evolution of the SV of C is analyzed as a function of depth and exposure conditions. SCAP-1D is then applied to horizontal trajectories of measurements made on the same face and at the same embedding depth z. The estimation of geostatistical parameters for each trajectory then allows plotting their evolutions as functions of z and exposure face. This study principle is illustrated in Figure 9.

5.3. Average diffusion parameters at 28 years

normally distributed random fields.

The 37 total chloride profiles were post-processed to determine the associated Fick's second law average diffusion parameters at the reference time t_r of 28 years, denoted C_{sa} and D_a . The fitting procedure to the peak of the profiles is described in (Othmen et al., 2018) and implemented in (Schoefs et al., 2023). Thus, one trajectory for each diffusion parameter per side is constructed at a height of 30 cm. These include 16 points for the Ext Side and 21 points for the Int Side, with a minimum distance of 30 cm between measurement points.

Table 6 summarizes this information and Figure 10 shows the trajectories. Note that the log function is applied to each parameter to (i) simplify the representation and visual analysis as they are assumed to be lognormally distributed, (ii) use identification procedures (see section 4.2) available for

6. Spatial variability estimates of total chloride content in J-beam and physical understanding

6.1. Preamble: trajectories conditioning (kriging, debiasing)

Taking into account the non-usable or missing measurements per sample and in order to take into account the maximum number of points per trajectory, these are first enriched by ordinary kriging of the available data (Baillargeon, 2005, Chapter 4; Chilès & Delfiner, 2012, Chapter 3). The preliminary plot of the raw trajectories does not show any obvious non-stationarity (Figure 11), and the expectation of the associated RF is not known.

The kriging is then performed from the Weighted Least-Square Estimate (WLSE) identification on the semivariograms of the initial trajectories. We consider an exponential covariance model and the presence of an additive measurement error, identified by (Bonnet et al., 2017). In addition, (Bonnet et al., 2017) also identified a measurement bias $b = 0.37 \times 10^{-3}$ (kg/kg concrete) associated with the measurement protocol used and with human errors during its implementation. All the measurements are therefore debiased before kriging so that the data considered are now C + b. In order to limit the influence of the kriged data on the estimation of the SV of C, only the trajectories for which their proportion is less than 25% are considered. In total, 13 trajectories with 16 points can be considered on the Ext side ($z \in [2.5; 75]$ mm) and 12 trajectories with 21 points on the Int side ($z \in [2.5, 65]$ mm). The number of kriged data per trajectory is detailed in Table 1 in the Appendix, and the selected debiased trajectories are shown in Figure 12. Less than 25% of the data are kriged to 65 mm depth for both sides. We point out that due to the absence of measurements on the abscissae of the non-exploitable specimens, the trajectory grids are irregular, with 30% of measurements missing for the Int side and 53% for the Ext side. The performance of the SCAP-1D normality tests is therefore affected (Clerc et al., 2019), and special care must be taken to ensure the consistency of the estimation results.

6.2. Geostatistical pre-study and choice of parameters (Step I):

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We recall that the studied trajectories are debiased. We then denote \tilde{Z} the RF of C such that

$$\tilde{Z} = Z - b$$
 | $b = -0.37 \times 10^{-3}$ kg/kg concrete (4)

In addition, (Othmen et al., 2018) have previously determined that the marginal law of C is lognormal, and (Bonnet et al., 2017) have identified the "measurement error" introduced by the measurement protocol, but only for a mortar. This is distributed according to a generalized extremum law (GeV, (Johnson et al., 1995, p. 3)) with the parameters k = 0.16, $\sigma = 9.3 \times 10^{-5}$, and $\mu = -8.4 \times 10^{-5}$. (Schoefs et al., 2023) recently pointed out that the error model varies with chloride content. Consequently, the error should vary according to each trajectory. For simplicity, it is

- 419 considered constant in this paper and its standard deviation is an average of the potential standard
- 420 deviations (Schoefs et al., 2023).
- The best-fitting RF model for \tilde{Z} is then the sum of a log-normal RF Y and a GeV random variable
- 422 W such that, according to Equation (1), Z = Y + W.
- However, the joint PDF of such an RF is not explicit, and SCAP-1D cannot simply be applied.
- The small number of points per trajectory also precludes an estimation by WLSE on semivariograms
- 425 (see 4.1.2).
- 426 Two modeling compromises are then possible in order to ensure explicitness:
- <u>Model a</u>: consider a normal marginal distribution of the total chloride content and a normal
- 428 distribution of the error, so that

$$\tilde{Z} \sim \underbrace{N(\mu(x), s. R(a))}_{G^1} + t. N(0, I)$$
 (5)

- where s and t are respectively the empirical variance of the signal and the error and R(a) is the
- 430 autocorrelation matrix.
- Model b: do not consider the measurement error, whose standard deviation $(1.55 \times 10^{-4} \text{ kg/kg})$
- concrete according to the model of (Bonnet et al., 2017)) would induce a maximum CoV of 8.3%
- 433 (on trajectory 13; Ext side, z = 75 mm). For this model,

$$\tilde{Z} \sim \exp\left(\underbrace{N(\mu(x), s. R(a))}_{G^2}\right)$$
 (6)

- 434 G^1 and LN are then the RF describing the SV of C, with G^2 being the source GRF of LN.
- Since the first approximation of the maximum CoV according to the model of (Bonnet et al., 2017)
- 436 is significant, we then apply SCAP-1D considering both models, so that:
- if the Signal-to-Noise Ratio (SNR) associated with model a is low, the error is not negligible and
- 438 model *a* is retained;
- if the SNR associated with model a is high, the error is considered negligible and model b is
- retained.

Due to the absence of non-stationarities visible in Figure 12, the trajectories are *a priori* assumed to be stationary, so that $\mu(x) = \mu$. We also recall that the model of the autocorrelation function chosen for G^1 and LN is exponential. According to (Chilès & Delfiner, 2012, p. 106), to guarantee such a model for the autocorrelation function $\rho_{LN}(a)$ of LN, the autocorrelation function ρ_{G^2} of G^2 must be written as follows

$$\rho_{C^2}(\cdot) = \ln(\rho_{IN}(\cdot), [e-1] + 1) \tag{7}$$

- 446 According to (Chilès & Delfiner, 2012, p. 106), the mean m_{LN} and the variance σ_{LN}^2 of LN are
- 447 then written as follows

$$m_{LN} = \exp\left(\mu + \frac{1}{2}s\right)$$
 (8); $\sigma_{LN}^2 = m_{LN}^2(\exp(s) - 1)$ (9)

- 448 6.3. Model a: Parameter estimation, hypothesis testing and analysis (Steps II, III
- 449 *and IV*)
- 450 Preliminary note: Prior to estimation, the trajectory data, of order 10^{-3} , are multiplied by a factor
- 451 $k = 10^4$ to facilitate the convergence of the MLE. The estimates of t, s and μ are then transformed
- 452 accordingly before analysis.
- Table 2 in the Appendix details the results of the SCAP-1D implementation considering model a.
- Only the results of the Σ_{mod} -tests are considered due to the small number of points per trajectory.
- Analyzing these, the estimates for trajectories 4 and 5 are rejected due to the rejection of the
- stationarity hypothesis by the KPSS-test and because the estimated error and practical range are
- zero and well below $2\Delta_x$, respectively, so that the spatial correlation of the data cannot be described
- 458 (see 4.1.5). This is also the case for trajectories 2, 11, 12, and 13 and can be explained by their very
- small number of points. Similarly, the estimates associated with trajectories 15, 17, and 24 are not
- considered because the estimated practical range is zero. Finally, no valid and convergent estimate
- can be obtained for trajectory 14. As a first step, for each selected trajectory, we plot the estimates

 $\hat{\mu}$ of the mean with their 95% MLE-based confidence intervals in Figure 13 (a)⁵, and the estimates 462 $\sqrt{\hat{s} + \hat{t}}$ of the standard deviation, for which the distribution is unknown, in Figure 13 (b). We then 463 464 note that these are consistent with the statistical mean and standard deviation profiles previously plotted in Figure 8: the values estimated by kriging (see 4.1) do not produce perturbations of these 465 statistical estimates. 466 As a second step, we plot in Figure 14 (a) the standard deviation estimates $\hat{\tau} = \sqrt{\hat{t}}$ and $\hat{\sigma} = \sqrt{\hat{s}}$ of 467 the error and signal, respectively, and the standard deviation τ_{ε} associated with the error model of 468 469 (Bonnet et al., 2017). Trajectories for which the estimated error is zero are not considered. We then compare the roots of the experimental SNRs \hat{t}/\hat{s} and the fixed error SNRs $\tau_{\varepsilon}^2/\hat{s}$ in Figure 14 (b). 470 Thus, we find that 75% of the roots of the experimental SNRs are greater than 100 and show no 471 472 specific trend along. The fixed error SNRs are logically lower than the experimental ones 473 because the error was calculated for mortars, for which the evaluation error is larger (Schoefs et 474 al., 2023). 475 Therefore, in accordance with the previously stated modeling assumptions, we consider the 476 measurement error to be negligible and retain model b for the SCAP-1D trajectory study.

6.4. Model b: Parameter estimation, hypothesis testing and analysis (Steps II, III, IV)

478 Preliminary note: Since the RF considered in model b is log-normal, the data considered for the

SCAP-1D implementation are the logarithms of the debiased data to consider a normal joint PDF.

480 The estimators of m_{LN} , σ_{LN}^2 and $r_{95,LN}$ are then calculated from Equations (7) to (9) and from the

estimators ŝ, t̂ and â.

Table 3 in the Appendix shows the result of the SCAP-1D implementation considering model *b*. Only

the results of the Σ_{mod} -tests are considered due to the small number of points per trajectory. The

analysis shows that the estimates for trajectories 3, 4, 5, and 16 are rejected due to the rejection of the

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⁵ some values are missing due to the singularity of the Fisher matrix

- stationarity hypothesis by the KPSS-test. A model of the mean and covariance function that satisfies
- 486 the hypothesis cannot be found, so no geostatistical parameter estimates are available for these
- 487 trajectories. 77% of the estimates are then valid on the Ext side (10/13) and 92% on the Int side
- 488 (11/12).
- The estimates \hat{m}_{LN} and $\hat{\sigma}_{LN}$ of the means and standard deviations are plotted with their confidence
- 490 intervals (CIs) for each trajectory in Figure 15. Note that the estimates are consistent with the
- 491 statistical mean and standard deviation profiles previously plotted in Figure 8. Meanwhile, the
- 492 estimates $\hat{r}_{95,LN}$ of the practical ranges are plotted with their CIs in Figure 16.
- 493 In the case of the trajectories associated with the Ext side, only 8 out of 10 estimates retained by
- 494 SCAP-1D (i.e., 47% of all estimates) are finally retained after filtering out the outliers. These
- estimates all satisfy the interpretability criterion (see 4.1.5) with values between 100 and 300 cm for
- 496 a minimum measurement step of 30 cm. However, contrary to the assumptions made at the beginning
- of the study, there is no trend in the evolution of $\hat{r}_{95,LN}$ as a function of the depth.
- In the case of the trajectories associated with the Int side, all the estimates retained by SCAP-1D can
- be considered after filtering out the outliers (i.e., 92% of all the estimates). In accordance with the
- 500 hypotheses formulated at the beginning of the study, we observe a practical range of zero near the
- surface, then a quasi-constant value around 60 cm-100 cm from 20 mm depth, respecting the
- interpretability criterion. This tendency reflects a transition from a purely random quantity of interest
- 503 (QI) at the surface to a spatially correlated QI. at depth. We propose the following explanation:
- At the surface, many spatiotemporally random material (microcracks, ...), physicochemical (local
- runoff, ...), and environmental phenomena (biofouling, ...) contribute to the supply and
- convection of chlorides, so that, in coherence with the central limit theorem, the chloride
- concentration at the surface can be considered completely random, at least at the scale of the 30
- 508 cm spatial discretization studied;
- at depth, the phenomenon of diffusion and fixation of C gradually takes precedence over their
- distribution within the coating, so that the fixation and diffusion properties of the concrete become

511 predominant with respect to the external and surface phenomena. Since the concrete has a certain 512 spatial homogeneity, it is then consistent that *C* is spatially correlated.

Furthermore, it is noted that the value of $\hat{r}_{95,LN}$ at the rebar depth is of the same order as the as the conventional spacing of the vibrations during concrete pouring, from 30 to 60 cm (Guiraud, 2018; SETRA, 2005, p. 199). This observation then reinforces the idea that the spatial correlation of C is strongly related to the homogeneity properties of the concrete.

6.5. Model b: construction of a unique autocorrelation model of chloride content

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The results obtained from the study of the Int side trajectories are much more convincing than those obtained from the study of the Ext side trajectories. In order to propose a unique autocorrelation model of C for the diffusion region, we then perform an MLE on the set of reduced centered trajectories of the Int side (without the first 10 mm), considering a unique scaling parameter a. Due to the non-stationarity of the mean and variance of C as a function of depth, we consider the trajectories independent of each other (we therefore neglect the SV along the z-axis). Still considering an exponential covariance model, the estimated practical range is $\hat{r}_{95} = 71$ cm, i.e. $\hat{a} = 24$ cm. We then state that the Gaussian model of identical practical range has the scale parameter $\hat{a} = 41$ cm. However, the calculation of the ER from the AICC does not allow us to distinguish between the two models from the data, either by considering each trajectory or by estimating the MLE over all trajectories. In fact, their values are between 1 and 1.29. They are reported in Table 4 in the Appendix. Considering only the estimation results from the Int side, which are considered more robust due to the larger amount of data per trajectory, we can partially answer the questions raised in the introduction of this study. The SV of the chloride content seems indeed to depend on the depth, with a large variability within the convection zone, and much less and more stable in the diffusion zone with a practical range of about 70 cm. However, this value is more than two to three times lower than the practical range values of the average diffusion parameters C_{sa} and D_a reported in the literature, between 176 and 264 cm (Engelund & Sørensen, 1998; Karimi, 2002; O'Connor &

Kenshel, 2013). However, it should be noted that the latter are obtained with less robust methods than SCAP-1D and with even smaller data sets. However, the question of the influence of the exposure conditions on the SV of the chloride content remains open due to the inconsistency of the practical range estimates on the Ext side. However, given the interpretation of the results for the Int side, it can be expected to have an influence only near the surface, where the chloride contents are already spatially decorrelated. However, this suggestion needs to be confirmed by studying an even richer database in order to increase the robustness of the estimates.

7. Spatial variability estimation of chloride diffusion parameters of J-beam and physical analysis

7.1. Geostatistical pre-study and choice of procedure parameters (Step I):

- The trajectories of $log(C_{sa})$ and $log(D_a)$ are shown in Figure 17 to Figure 20, as well as their means and the histograms and experimental semivariogams of centered trajectories. These curves are plotted for each trajectory in three cases: (i) stationary mean (no changepoint); (ii) piecewise stationary mean with one changepoint determined via the PELT algorithm (Killick et al., 2012); (iii) piecewise stationary average at two changepoints determined via the PELT algorithm.

 The presence of sills on the experimental semi-variograms allows identifying the *a priori* stationary centered trajectories and thus the most appropriate non-stationarity model for each. The shapes of the histograms and semi-variograms (nugget effect, tangent to the origin, ...) allows identifying models of joint density and a priori autocorrelation function that are adequate to the geostatistical description of the data.
- Since the amount of data per trajectory is limited, especially for the Ext side, Step I could only be executed for the trajectories $\log(C_{sa})$ 1 and $\log(D_a)$ 1. The observations made as well as the models considered *a priori* are presented in Table 7.

- 560 7.1.1. Fields models
- Although the histogram of the centered trajectory $log(C_{sa})$ 1 is asymmetric, this may be due to
- the spatial correlation of the data. The histogram of $log(D_a)$ 1 is centered. Moreover, we do not
- notice any nugget effect. This is due to the fact that the measurement noise of the chloride data is
- negligible (see 6.3). For simplicity and consistency with the results of the literature, a Gaussian
- Random Field model (GRF) is used to estimate spatial variability. This model is also retained by
- default for the $\log(C_{sa})$ 2 and $\log(D_a)$ 2 trajectories.
- 567 7.1.2. Averaging models: apparent non-stationarity
- The trajectories are *a priori* globally stationary since the experimental semi-variograms without any
- changepoint in the mean present a sill. A constant mean model is therefore retained.
- 570 7.1.3. Covariance function models
- Due to the large between measurement points, the spatial resolution is insufficient to visualize the
- 572 presence of a tangent at the origin of the experimental semi-variograms. We therefore retain both the
- Gaussian and exponential covariance models without *a priori* preference.
- This choice does not allow accounting for the correlation between $log(C_{sa})$ and $log(D_a)$, well
- identified by (Othmen et al., 2018) and which should be modeled via a cross-correlated Random Field
- 576 (XRF) (Chilès & Delfiner, 2012, p. 332). It is however justified as XRF modelling requires additional
- 577 parameters (cross-correlation coefficient and cross-scale parameter(s)) which leads to
- overparameterization and unsuccessful assessment attempts due to the limited amount of data⁶.
- 579 7.2. Parameter estimation, hypothesis testing and analysis (Steps II, III and IV)
- SCAP-1D steps II and III are applied to the trajectories of $log(D_a)$ considering the models kept in
- 7.1. This allows the most appropriate covariance model to be determined from the calculation of

⁶ MLE on raw data and MLE on data enriched via Metropolis-Hastings sampling did not give satisfactory results

582 Evidence Ratios (ER). These are computed from the AICC. Table 8 shows the results of SCAP-1D 583 execution by trajectory. The models are ranked by increasing value of their ER. 584 First no estimate is rejected by the hypothesis tests. The estimates computed from the exponential and Gaussian models are then consistent for each of the trajectories except for the trajectory $\log(C_{sq})$ 585 2 (relative errors of respectively 21%, 63%, 9% and 21% between the practical ranges of the 586 587 Gaussian model and those of the exponential model). Furthermore, the exponential and Gaussian 588 covariance models cannot be distinguished from the ERs (maximum difference observed for the 589 $\log(C_{sa})$ 1 trajectory with a ratio of 1 for the Gaussian model and a ratio of 2.13 for the 590 exponential model). However, the Gaussian model leads to smaller coefficients of variation of the geostatistical parameters (up to a factor of 2.5 for the scale parameter of trajectory $\log(C_{sa})$ 1). 591 592 Second, the majority of the practical range estimates do not meet the interpretability criterion (see 593 4.1.5). All of the estimates of r_{95} made on the $\log(D_a)$ trajectories as well as the estimate made on the $\log(C_{sq})$ 2 trajectories are indeed of the order of 30 cm, which is the minimum spacing 594 595 between measurements. 596 Therefore, only the estimates made on the $log(C_{sa})$ 1 trajectory are retained. The estimated practical 597 range is indeed 50 to 63 cm depending on the chosen covariance model, exponential or Gaussian (i.e., a ratio r_{95}/L of 0.05 to 0.07). The latter can then be preferred a priori because of its minimal 598 599 ER even if exponential model is close. 600 Thus, due to the too large spacing between measurement points compared to the hidden value of their practical range, the question of whether the SV of the scattering parameters is a function of the 601 602 exposure conditions cannot be answered here. 603 Furthermore, the interpretable estimates of practical ranges differ significantly from the 100 to 200 604 cm values estimated by (O'Connor & Kenshel, 2013) on a bridge exposed to chloride ion penetration. 605 Such a difference can be explained by the fact that these authors significantly enriched their initial kriging database (number of points multiplied by 7) before producing their estimates, which may have 606 607 influenced them significantly.

8. Conclusion

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- In this paper, a new database is proposed for analysis of the spatial variability of chloride ingress into concrete. A total of 460 measurements was available, belonging to 2 exposures zones: 12 trajectories of 21 points for each profile on the Internal side (sheltered from wetting-drying cycles) and 13 trajectories of 16 points on the External side. This data base was uses to analyze the spatial variability of (i) chloride content, (ii) parameters of a model of chloride diffusion. The analysis is performed on the basis of a complete framework (SCAP-1D) which raises, rigorously for the first time, questions of stationarity, marginal distribution and auto-correlation.
- The application to the total chloride content data showed that:
- the measurement error on total chloride content is negligible for the estimation of their spatial variability;
- near the surface, spatio-temporally random physico-chemical and environmental phenomena contribute to the diffusion and convection of chlorides so that, in coherence with the central limit theorem, total chloride content at the surface can be considered totally random;
- in depth (from about 20 mm), total chlorides diffusion and fixation gradually take precedence 622 623 over their distribution within the coating, so that fixation and diffusion properties of concrete 624 become predominant compared to the external and surface phenomena. Total chloride content is 625 then spatially correlated with an estimated general practical range of 71 cm (scale parameter of 626 24 cm) which is of the same order as the conventional vibration spacing during concrete pouring 627 (30-60 cm (Guiraud, 2018; SETRA, 2005, p. 199)). This supports the fact that total chloride 628 content spatial correlation in depth is strongly related to the homogeneity properties of the 629 concrete resulting from pouring.
 - The application to the identification of diffusion parameters showed that:
- The application to the chloride diffusion parameters data was presented in section 7. It allowed estimation of geostatistical parameters only for the logarithm of the average surface chlorides concentration. This is due to a limited number of points per trajectory (from 16 to 21) and a too large

spacing between measurement points (30 cm). The estimated practical range value is nevertheless consistent with that of the total chloride content in depth since it is between 50 and 63 cm. Implementation of SCAP-1D on these two datasets allowed to note that when the number of points per trajectory is limited but not small (\geq 20), it is more efficient to first consider the results of statistical tests performed on the standardized trajectory from the empirical covariance matrix, and then in a second step those performed on the standardized trajectory from the modeled covariance matrix. This allows to first assess the validity of the mean model, then those of the marginal density model and the estimates of the geostatistical parameters of the source Random Field. On the contrary, when the number of points per trajectory is small (< 20), it is preferable to consider only the results of the statistical tests performed on the standardized trajectory from the modeled covariance matrix, in order to guarantee its positivity of the latter.

10. Appendix – Detailed tables of estimation process data

Table 1- Definition of trajectories and proportion of kriged data per trajectory – Ext side and Int side

	kriged data -	Ex side)		kriged data	- Int side	
trajectory	depth (mm)	number	percentage	trajectory	depth (mm)	number	percentage
1	2.5	0	0%	16	2.5	0	0%
2	7.5	0	0%	17	7.5	1	5%
3	12.5	0	0%	18	12.5	1	5%
4	17.5	0	0%	19	17.5	1	5%
5	22.5	0	0%	20	22.5	2	10%
6	27.5	0	0%	21	27.5	1	5%
7	32.5	0	0%	22	32.5	2	10%
8	37.5	2	13%	23	37.5	2	10%
9	42.5	3	19%	24	42.5	4	19%
10	47.5	4	25%	25	47.5	3	14%
11	55	1	6%	26	55	4	19%
12	65	1	6%	27	65	4	19%
13	75	3	19%	28	75	9	43%
14	85	9	56%	29	85	15	71%
15	95	10	63%	30	95	17	81%
	Total	33	14%		Total	66	21%

Table 2 - Estimation of geostatistical parameters of total chloride concentration trajectories by SCAP-1D - model a

	trajectory model						Σexp-tests				Σmod-tests				estimates (data multiplied by k=10e4)					Coefficient of Variation (CoV)				
#	Side	z (mm)	Npts	model	cov	reg	chg	chi2	KS	KPSS	DF	chi2	KS	KPSS	DF	μ	s	t	а	r95	CoV μ	CoV s	CoV t	CoV a
1	Ext	2,5	16	GW	expon	0	0	0	0	0	1	0	0	0	1	18,66	19,63	0,00	0,01	0,03	NaN	NaN	NaN	NaN
2	Ext	7,5	16	GW	expon	0	0	0	0	0	1	0	0	0	1	25,86	64,36	0,00	0,00	0,00	2,0E-01	1,2E+00	5,7E-01	1,7E+00
3	Ext	12,5	16	GW	expon	0	0	1	0	1	0	0	0	0	1	40,04	49,27	36,47	2,57	6,28	1,2E-01	8,9E-01	5,3E-01	1,3E+00
4	Ext	17,5	16	GW	expon	0	0	0	0	0	1	0	0	- 1	1	41,59	91,14	0,00	0,01	0,01	5,7E-02	7,6E+02	1,8E+02	3,8E+05
5	Ext	22,5	16	GW	expon	0	0	0	0	1	0	0	0	- 1	1	39,44	96,31	0,00	0,01	0,01	NaN	NaN	NaN	NaN
6	Ext	27,5	16	GW	expon	0	0	1	0	0	1	0	0	0	1	36,65	53,47	0,00	0,52	1,55	NaN	NaN	NaN	NaN
7	Ext	32,5	16	GW	expon	0	0	1	0	0	1	0	0	0	1	34,95	59,57	0,00	0,32	0,96	NaN	NaN	NaN	NaN
8	Ext	37,5	16	GW	expon	0	0	0	0	0	1	0	0	0	1	30,95	62,31	0,00	0,37	1,12	NaN	NaN	NaN	NaN
9	Ext	42,5	16	GW	expon	0	0	0	0	0	1	0	0	0	1	29,20	18,89	7,15	2,17	5,80	9,1E-02	7,5E-01	7,5E-01	1,1E+00
10	Ext	47,5	16	GW	expon	0	0	0	0	0	1	0	0	0	1	27,69	19,49	0,00	0,71	2,12	NaN	NaN	NaN	NaN
11	Ext	55	16	GW	expon	0	0	0	0	0	1	0	0	0	1	20,98	16,46	0,00	0,01	0,02	NaN	NaN	NaN	NaN
12	Ext	65	16	GW	expon	0	0	0	1	1	0	0	0	0	1	18,89	11,95	0,00	0,01	0,03	4,6E-02	1,5E+02	9,6E+09	5,6E+05
13	Ext	75	16	GW	expon		0	0	0	0	1	0	0	0	1	18,61	8,47	0,00	0,01	0,03	3,9E - 02	1,9E+02	9,0E+09	1,5E+05
14	Int	2,5	21	GW	expon	0	0	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN
15	Int	7,5	21	GW	expon	0	0	0	0	0	1	0	0	0	1	22,94	135,81	0,00	0,00	0,00	NaN	NaN	NaN	NaN
16	Int	12,5	21	GW	expon	0	0	1	1	0	1	0	0	0	1	40,27	38,35	111,41	3,19	5,21	1,2E-01	1,3E+00	3,6E-01	1,5E+00
17	Int	17,5	21	GW	expon	0	0	0	0	0	1	0	0	0	1	53,50	180,15	0,00	0,00	0,00	NaN	NaN	NaN	NaN
18	Int	22,5	21	GW	expon	0	0	0	0	0	1	0	0	0	1	55,01	150,55	0,00	0,34	1,01	6,5E-02	1,1E+00	1,8E+07	1,0E+00
19	Int	27,5	21	GW	expon	0	0	0	0	0	1	0	0	0	1	50,09	154,45	0,00	0,24	0,72	6,5E-02	1,8E+00	3,2E+05	1,1E+00
20	Int	32,5	21	GW	expon		0	0	0	0	1	0	0	0	1	47,18	168,62	0,00	0,31	0,93	7,8E-02	9,9E-01	8,2E+07	8,8E-01
21	Int	37,5	21	GW	expon		0	0	0	0	1	0	0	0	1	39,46	126,85	4,31	0,09	0,26	6,4E-02	2,6E+01	7,8E+02	7,8E+00
22	Int	42,5	21	GW	expon		0	0	0	1	0	0	0	0	1	39,57	103,23	2,06	0,29	0,86	7,2E-02	1,2E+00	5,9E+01	1,1E+00
23	Int	47,5	21	GW	expon		0	1	0	0	1	0	0	0	1	34,59	72,53	0,00	0,35	1,06	7,3E - 02	9,4E-01	2,7E+07	8,5E-01
24	Int	55	21	GW	expon	0	0	0	0	0	1	0	0	0	1	26,46	28,31	0,00	0,00	0,00	NaN	NaN	NaN	NaN
25	Int	65	21	GW	expon	0	0	0	0	0	1	0	0	0	1	24,73	25,35	0,00	0,25	0,74	5,4E-02	1,9E+00	6,9E+05	1,2E+00

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	trajectory				model					Σexp-tests				Σmod-tests			s (logarithn	of data)	Coefficients of Variation (CoV)		
#	Face	z (mm)	Npts	modèle	cov	reg	chg	chi2	KS	KPSS	DF	chi2	KS	KPSS	DF	μ	s	а	CoV μ	CoV s	CoV a
1	Ext	2,5	16	G	expon	0	0	0	1	0	1	0	0	0	1	6,31	0,05	0,00	NaN	NaN	NaN
2	Ext	7,5	16	G	expon	0	0	0	1	1	1	0	0	0	1	6,04	0,08	0,52	1,6E-02	4,2E-01	7,5E-01
3	Ext	12,5	16	G	expon	0	0	0	0	0	1	0	0	1	1	5,54	0,06	0,26	1,3E-02	3,7E-01	8,4E-01
4	Ext	17,5	16	G	expon	0	0	0	0	0	1	0	0	1	1	5,51	0,05	0,00	1,0E-02	3,5E-01	1,8E+06
5	Ext	22,5	16	G	expon	0	0	0	0	1	1	0	0	1	1	5,57	0,07	0,13	1,2E-02	3,5E-01	1,6E+00
6	Ext	27,5	16	G	expon	0	0	0	0	1	1	0	0	0	1	5,63	0,05	0,47	1,3E-02	4,0E-01	6,4E-01
7	Ext	32,5	16	G	expon	0	0	0	1	1	1	0	0	0	1	5,68	0,05	0,25	1,1E-02	3,7E-01	9,4E-01
8	Ext	37,5	16	G	expon	0	0	0	0	0	1	0	0	0	1	5,81	0,06	0,40	1,4E-02	3,9E-01	6,7E-01
9	Ext	42,5	16	G	expon	0	0	0	1	1	0	0	0	0	1	5,86	0,03	0,93	1,3E-02	4,8E-01	6,5E-01
10	Ext	47,5	16	G	expon	0	0	0	1	1	1	0	0	0	1	5,90	0,03	0,69	1,1E-02	4,6E-01	7,2E-01
11	Ext	55	16	G	expon	0	0	0	0	0	1	0	0	0	1	6,19	0,05	0,02	8,7E-03	3,5E-01	4,8E+02
12	Ext	65	16	G	expon	0	0	0	0	0	1	0	0	0	1	6,29	0,04	0,01	NaN	NaN	NaN
13	Ext	75	16	G	expon	0	0	0	0	0	1	0	0	0	1	6,30	0,03	0,01	6,5E-03	3,5E-01	2,0E+05
14	Int	2,5	21	G	expon	0	0	0	0	0	1	0	0	0	1	6,18	0,05	0,00	NaN	NaN	NaN
15	Int	7,5	21	G	expon	0	0	0	0	0	1	0	0	0	1	6,19	0,21	0,00	NaN	NaN	NaN
16	Int	12,5	21	G	expon	0	0	0	0	0	1	0	0	1	1	5,58	0,14	0,01	1,5E-02	3,1E-01	1,5E+05
17	Int	17,5	21	G	expon	0	0	0	0	0	1	0	0	0	1	5,26	0,06	0,16	1,1E-02	3,1E-01	7,8E-01
18	Int	22,5	21	G	expon	0	0	0	1	0	1	0	0	0	1	5,23	0,05	0,34	1,2E-02	3,4E-01	5,3E-01
19	Int	27,5	21	G	expon	0	0	0	1	0	1	0	0	0	1	5,32	0,05	0,28	1,2E-02	3,3E-01	5,8E-01
20	Int	32,5	21	G	expon	0	0	0	1	0	1	0	0	0	1	5,39	0,06	0,28	1,3E-02	3,3E-01	6,0E-01
21	Int	37,5	21	G	expon	0	0	0	1	0	1	0	0	0	1	5,58	0,08	0,13	1,2E-02	3,1E-01	1,0E+00
22	Int	42,5	21	G	expon	0	0	0	0	0	1	0	0	0	1	5,57	0,07	0,28	1,3E-02	3,3E-01	5,7E-01
23	Int	47,5	21	G	expon	0	0	0	1	0	1	0	0	0	1	5,70	0,06	0,30	1,2E-02	3,3E-01	5,7E-01
24	Int	55	21	G	expon	0	0	0	0	0	1	0	0	0	1	5,95	0,04	0,19	7,9E-03	3,1E-01	6,4E-01
25	Int	65	21	G	expon	0	0	0	0	0	1	0	0	0	1	6,02	0,04	0,24	8,4E-03	3,2E-01	5,9E-01

G : Gaussian; reg : regression degree of mean model; chg : maximum number of changepoints of mean model

Table 4 - Computation of the evidence ratios associated with the exponential and Gaussian covariance models for model b using the corrected Akaike criterion

	tr	ajectory		n	nodel		Corrected Al	kaike criteria	Evidence Ratios		
#	Face	z (mm)	Npts	model	reg	chg	cov gauss	cov expon	cov gauss	cov expon	
17	Int	17.5	21	G	0	0	-3.21E+01	-3.22E+01	1.04	1.00	
18	Int	22.5	21	G	0	0	-3.89E+01	-3.89E+01	1.00	1.05	
19	Int	27.5	21	G	0	0	-3.59E+01	-3.55E+01	1.00	1.21	
20	Int	32.5	21	G	0	0	-3.05E+01	-3.07E+01	1.11	1.00	
21	Int	37.5	21	G	0	0	-2.39E+01	-2.39E+01	1.00	1.01	
22	Int	42.5	21	G	0	0	-2.89E+01	-2.94E+01	1.29	1.00	
23	Int	47.5	21	G	0	0	-3.14E+01	-3.18E+01	1.18	1.00	
24	Int	55	21	G	0	0	-4.21E+01	-4.18E+01	1.00	1.14	
25	Int	65	21	G	0	0	-4.22E+01	-4.18E+01	1.00	1.26	

G: gaussian

11. Data Availability Statement 658 Some or all data, models, or code generated or used during the study are available from the 659 660 corresponding author by request: chloride content dataset; 661 662 • whole MATLAB code generated during the study. 663 12. Acknowledgment The authors would like to thank all the partners of iMAREO2 project: Keops Automation (D. Follut, 664 D. Olivier), Université de Nantes (M. Roche), Nantes – Saint-Nazaire Port (P. Lijour, M. Labegorre). 665 666 The authors would like to thank the Pays de la Loire region for its financial support. 667 13. References Angst, U., Elsener, B., Larsen, C. K., & Vennesland, Ø. (2009). Critical chloride content in 668 669 reinforced concrete—A review. Cement and Concrete Research, 39(12), 1122–1138. 670 https://doi.org/10.1016/j.cemconres.2009.08.006 671 Arnaud, M., & Emery, X. (2000). Estimation et interpolation spatiale: Méthodes déterministes et 672 méthodes géostatistiques. Hermès. http://agritrop.cirad.fr/487412/ 673 Baillargeon, S. (2005). Le Krigeage. Faculté des sciences et de génie UNIVERSITÉ LAVAL 674 QUÉBEC. https://archimede.mat.ulaval.ca/theses/S-Baillargeon_05.pdf 675 Bastidas-Arteaga, E., & Schoefs, F. (2015). Sustainable maintenance and repair of RC coastal 676 structures. *Proceedings of the ICE-Maritime Engineering*, 168(4), 162–173. 677 Biondi, L., Perry, M., McAlorum, J., Vlachakis, C., Hamilton, A., & Lo, G. (2021). Alkali-678 Activated Cement Sensors for Sodium Chloride Monitoring. *IEEE Sensors Journal*, 21(19), 679 21197–21204. https://doi.org/10.1109/JSEN.2021.3100582 680 Bonnet, S., Schoefs, F., & Salta, M. (2017). Sources of uncertainties for total chloride profile 681 measurements in concrete: Quantization and impact on probability assessment of corrosion 682 initiation. European Journal of Environmental and Civil Engineering, 1–16. 683 https://doi.org/10.1080/19648189.2017.1375997 684 Bourreau, L., Bouteiller, V., Schoefs, F., Gaillet, L., Thauvin, B., Schneider, J., & Naar, S. (2019). 685 Uncertainty assessment of concrete electrical resistivity measurements on a coastal bridge. 686 *Structure and Infrastructure Engineering*, 1–11. 687 https://doi.org/10.1080/15732479.2018.1557703

- Bourreau, L., Gaillet, L., Bouteiller, V., Schoefs, F., Thauvin, B., Schneider, J., & Naar, S. (2020).
- Spatial identification of exposure zones of concrete structures exposed to a marine
- environment with respect to reinforcement corrosion. Structure and Infrastructure
- Engineering, 16(2), 346–354. https://doi.org/10.1080/15732479.2019.1655072
- BS 6349-1:2000. (2000). Maritime structures. Code of practice for general criteria [Norme].
- 693 https://shop.bsigroup.com/ProductDetail/?pid=00000000030055558
- Burnham, K. P., & Anderson, D. R. (2010). Model selection and multimodel inference: A practical
- *information-theoretic approach* (2. ed., [4. printing]). Springer.
- 696 Chilès, J.-P., & Delfiner, P. (2012). *Geostatistics: Modeling spatial uncertainty* (2. ed). Wiley.
- 697 Clerc, R. (2021). Sur l'estimation de la variabilié spatiale des paramètres de corrosion et sa
- 698 necessité pour les plans de maintenance des ouvrages maritimes en béton armé [These de
- doctorat, Nantes]. https://www.theses.fr/fr/2021NANT4052
- 700 Clerc, R., Oumouni, M., & Schoefs, F. (2019). SCAP-1D: A Spatial Correlation Assessment
- Procedure from unidimensional discrete data. Reliability Engineering & System Safety, 191,
- 702 106498. https://doi.org/10.1016/j.ress.2019.106498
- 703 Cressie, N. A. C. (1993). Statistics for spatial data (Rev. ed). Wiley.
- Der Kiureghian, A., & Ke, J.-B. (1987). The stochastic finite element method in structural
- reliability. In Y. K. Lin, G. I. Schuëller, & P. Spanos (Eds.), *Stochastic Structural*
- 706 *Mechanics* (pp. 84–109). Springer Berlin Heidelberg.
- 707 Dickey, D. A., & Fuller, W. A. (1981). Likelihood Ratio Statistics for Autoregressive Time Series
- 708 with a Unit Root. *Econometrica*, 49(4), 1057. https://doi.org/10.2307/1912517
- 709 Engelund, S., & Sørensen, J. D. (1998). A probabilistic model for chloride-ingress and initiation of
- 710 corrosion in reinforced concrete structures. *Structural Safety*, 20(1), 69–89.
- Fares, M., Villain, G., Bonnet, S., Palma Lopes, S., Thauvin, B., & Thiery, M. (2018). Determining
- 712 chloride content profiles in concrete using an electrical resistivity tomography device.
- 713 *Cement and Concrete Composites*, 94, 315–326.
- 714 https://doi.org/10.1016/j.cemconcomp.2018.08.001
- Glasser, F. P., Marchand, J., & Samson, E. (2008). Durability of concrete—Degradation phenomena
- involving detrimental chemical reactions. Cement and Concrete Research, 38(2), 226–246.
- 717 https://doi.org/10.1016/j.cemconres.2007.09.015
- Gomez-Cardenas, C., Sbartaï, Z. M., Balayssac, J. P., Garnier, V., & Breysse, D. (2015). New
- optimization algorithm for optimal spatial sampling during non-destructive testing of
- 720 concrete structures. *Engineering Structures*, 88, 92–99.
- 721 https://doi.org/10.1016/j.engstruct.2015.01.014

- Guiraud, P. (2018, April). Vibration des bétons. *Infociments*.
- https://www.infociments.fr/betons/vibration-des-betons
- Hill, R. (1963). Elastic properties of reinforced solids: Some theoretical principles. *Journal of the*
- 725 *Mechanics and Physics of Solids*, 11(5), 357–372. https://doi.org/10.1016/0022-
- 726 5096(63)90036-X
- Hunkeler, D. F., Ungricht, H., & Deillon, F. (2000). Determination of the chloride content of
- 728 concrete and execution of a Round Robin test in two steps (VSS 546). Eidg. Departement für
- 729 Umwelt, Verkehr, Energie und Kommunikation (UVEK), Bundesamt für Strassen.
- https://www.tfb.ch/Htdocs/Files/v/8671.pdf/Publikationsliste/ChloridbestimmungRingversu
- 731 chBerichtVSSNr.5462000.pdf
- Johnson, N. L., Balakrishnan, N., & Kotz, S. (1995). Continuous univariate distributions. Vol.2
- 733 (2ed.). Wiley.
- Karimi, A. R. (2002). Probabilistic assessment of deterioration and strength of concrete bridge
- 735 beams and slabs [Imperial College London].
- https://ethos.bl.uk/OrderDetails.do?uin=uk.bl.ethos.540519
- Kaushik, S. K., & Islam, S. (1995). Suitability of sea water for mixing structural concrete exposed
- to a marine environment. Cement and Concrete Composites, 17(3), 177–185.
- 739 https://doi.org/10.1016/0958-9465(95)00015-5
- Killick, R., Fearnhead, P., & Eckley, I. A. (2012). Optimal Detection of Changepoints With a
- Linear Computational Cost. *Journal of the American Statistical Association*, 107(500),
- 742 1590–1598. https://doi.org/10.1080/01621459.2012.737745
- Kwiatkowski, D., Phillips, P. C. B., Schmidt, P., & Shin, Y. (1992). Testing the null hypothesis of
- stationarity against the alternative of a unit root: How sure are we that economic time series
- have a unit root? *Journal of Econometrics*, 54(1), 159–178. https://doi.org/10.1016/0304-
- 746 4076(92)90104-Y
- Lecieux, Y., Rozière, E., Gaillard, V., Lupi, C., Leduc, D., Priou, J., Guyard, R., Chevreuil, M., &
- Schoefs, F. (2019). Monitoring of a Reinforced Concrete Wharf Using Structural Health
- Monitoring System and Material Testing. *Journal of Marine Science and Engineering*, 7(4),
- 750 84. https://doi.org/10.3390/jmse7040084
- 751 Lecieux, Y., Schoefs, F., Bonnet, S., Lecieux, T., & Lopes, S. P. (2015). Quantification and
- uncertainty analysis of a structural monitoring device: Detection of chloride in concrete
- using DC electrical resistivity measurement. *Nondestructive Testing and Evaluation*, 30(3),
- 754 216–232. https://doi.org/10.1080/10589759.2015.1029476

755	O'Connor, A. J., & Kenshel, O. (2013). Experimental Evaluation of the Scale of Fluctuation for
756	Spatial Variability Modeling of Chloride-Induced Reinforced Concrete Corrosion. Journal
757	of Bridge Engineering, 18(1), 3–14. https://doi.org/10.1061/(ASCE)BE.1943-5592.0000370
758	Othmen, I., Bonnet, S., & Schoefs, F. (2018). Statistical investigation of different analysis methods
759	for chloride profiles within a real structure in a marine environment. Ocean Engineering,
760	157, 96–107. https://doi.org/10.1016/j.oceaneng.2018.03.040
761	Oumouni, M., & Schoefs, F. (2021a). A Perturbed Markovian process with state-dependent
762	increments and measurement uncertainty in degradation modeling. Computer-Aided Civil
763	and Infrastructure Engineering, 36(8), 978–995. https://doi.org/10.1111/mice.12644
764	Oumouni, M., & Schoefs, F. (2021b). Spatial variability assessment of structures from adaptive
765	NDT measurements. Structural Safety, 89, 102052.
766	https://doi.org/10.1016/j.strusafe.2020.102052
767	Priou, J., Lecieux, Y., Chevreuil, M., Gaillard, V., Lupi, C., Leduc, D., Rozière, E., Guyard, R., &
768	Schoefs, F. (2019). In situ DC electrical resistivity mapping performed in a reinforced
769	concrete wharf using embedded sensors. Construction and Building Materials, 211, 244-
770	260. https://doi.org/10.1016/j.conbuildmat.2019.03.152
771	Qu, F., Li, W., Dong, W., Tam, V. W. Y., & Yu, T. (2021). Durability deterioration of concrete
772	under marine environment from material to structure: A critical review. Journal of Building
773	Engineering, 35, 102074. https://doi.org/10.1016/j.jobe.2020.102074
774	Ragab, A. M., Elgammal, M. A., Hodhod, O. A., & Ahmed, T. E. (2016). Evaluation of field
775	concrete deterioration under real conditions of seawater attack. Construction and Building
776	Materials, 119, 130-144. https://doi.org/10.1016/j.conbuildmat.2016.05.014
777	Ravahatra, N. R., Duprat, F., Schoefs, F., de Larrard, T., & Bastidas-Arteaga, E. (2017). Assessing
778	the Capability of Analytical Carbonation Models to Propagate Uncertainties and Spatial
779	Variability of Reinforced Concrete Structures. Frontiers in Built Environment, 3.
780	https://doi.org/10.3389/fbuil.2017.00001
781	Safhi, A. el M., Benzerzour, M., Rivard, P., Abriak, NE., & Ennahal, I. (2019). Development of
782	self-compacting mortars based on treated marine sediments. Journal of Building
783	Engineering, 22, 252–261. https://doi.org/10.1016/j.jobe.2018.12.024
784	Schoefs, F., Awa Zahui Raissa, K., Bonnet, S., & O'Conor, A. J. (2023). Uncertainty quantification
785	of semi-destructive testing for chloride content assessment for a concrete bridge in maritime
786	environment. Frontiers in Built Environment, 9.
787	https://www.frontiersin.org/articles/10.3389/fbuil.2023.1130066

788	Schoefs, F., Bastidas-Arteaga, E., Tran, T. V., Villain, G., & Derobert, X. (2016). Characterization
789	of random fields from NDT measurements: A two stages procedure. Engineering Structures.
790	111, 312–322. https://doi.org/10.1016/j.engstruct.2015.11.041
791	Schoefs, F., Bastidas-Arteaga, E., & Tran, TV. (2017). Optimal embedded sensor placement for
792	spatial variability assessment of stationary random fields. Engineering Structures, 152, 35-
793	44. https://doi.org/10.1016/j.engstruct.2017.08.070
794	Schoefs, F., Oumouni, M., Clerc, R., Othmen, I., & Bonnet, S. (2017). Statistical analysis and
795	probabilistic modeling of chloride ingress spatial variability in concrete coastal
796	infrastructures. Edition 4, Split, Croatie, 229–234. https://doi.org/10.5150/cmcm.2017.042
797	Schoefs, F., Oumouni, M., Follut, D., Lecieux, Y., Gaillard, V., Lupi, C., & Leduc, D. (2022).
798	Added value of monitoring for the maintenance of a reinforced concrete wharf with spatial
799	variability of chloride content: A practical implementation. Structure and Infrastructure
800	Engineering, $\theta(0)$, 1–13. https://doi.org/10.1080/15732479.2022.2077767
801	SETRA. (2005). Mémoar—Mémento pour la mise en oeuvre sur ouvrages d'art. Cerema (ex-Setra).
802	https://www.cerema.fr/fr/centre-ressources/boutique/memoar-memento-mise-oeuvre-
803	ouvrages-art
804	Stewart, M. G. (2004). Spatial variability of pitting corrosion and its influence on structural fragility
805	and reliability of RC beams in flexure. Structural Safety, 26(4), 453-470.
806	https://doi.org/10.1016/j.strusafe.2004.03.002
807	Stewart, M. G., & Mullard, J. A. (2007). Spatial time-dependent reliability analysis of corrosion
808	damage and the timing of first repair for RC structures. Engineering Structures, 29(7),
809	1457–1464. https://doi.org/10.1016/j.engstruct.2006.09.004
810	Stewart, M. G., & Val, D. V. (2003). Multiple Limit States and Expected Failure Costs for
811	Deteriorating Reinforced Concrete Bridges. Journal of Bridge Engineering, 8(6), 405-415.
812	https://doi.org/10.1061/(ASCE)1084-0702(2003)8:6(405)
813	Torres-Luque, M., Bastidas-Arteaga, E., Schoefs, F., Sánchez-Silva, M., & Osma, J. F. (2014).
814	Non-destructive methods for measuring chloride ingress into concrete: State-of-the-art and
815	future challenges. Construction and Building Materials, 68, 68-81.
816	Torres-Luque, M., Osma, J. F., Sánchez-Silva, M., Bastidas-Arteaga, E., & Schoefs, F. (2017).
817	Chlordetect: Commercial Calcium Aluminate Based Conductimetric Sensor for Chloride
818	Presence Detection. Sensors, 17(9), 2099. https://doi.org/10.3390/s17092099
819	Vennesland, P. Ø., Climent, M. A., & Andrade, C. (2013). Recommendations of RILEM TC 178-
820	TMC: Testing and modelling chloride penetration in concrete. <i>Materials and Structures</i> , 8.
821	https://doi.org/10.1617/s11527-012-9968-1

822	wasserman, L. (2004). Att of Statistics. Springer New York. https://doi.org/10.1007/978-0-387-
823	21736-9
824	Watanabe, A., Tokuda, S., Mizuta, Y., Miyamoto, S., Nakanishi, T., Furukawa, H., & Minagawa,
825	H. (2021). Toward automated non-destructive diagnosis of chloride attack on concrete
826	structures by near infrared spectroscopy. Construction and Building Materials, 305, 124796.
827	https://doi.org/10.1016/j.conbuildmat.2021.124796
828	Wegian, F. M. (2010). Effect of seawater for mixing and curing on structural concrete. The IES
829	Journal Part A: Civil & Structural Engineering, 3(4), 235–243.
830	https://doi.org/10.1080/19373260.2010.521048
831	Zaki, A., Chai, H. K., Aggelis, D. G., & Alver, N. (2015). Non-Destructive Evaluation for
832	Corrosion Monitoring in Concrete: A Review and Capability of Acoustic Emission
833	Technique. Sensors, 15(8), Article 8. https://doi.org/10.3390/s150819069
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14. Embedded Tables

Table 5 - Exponential and Gaussian autocorrelation functions, and relations between their scale parameter, fluctuation scale and practical range

Name	Model	θ	$\rho(h=\theta)$	r_{95}	$\rho(h=r_{95})$
exponential	$\rho(h) = \exp\left(-\ h\ /a\right)$	2a	0.135	3a	0.05
Gaussian	$\rho(h) = \exp\left(-\ h\ ^2/a\right)$	$\sqrt{\pi}a$	0.043	1.73a	0.05

a: scale parameter; θ : fluctuation parameter; r_{95} : practical range;

 $\rho(\cdot)$: autocorrelation function

Table 6- Numbering of the trajectories of average scattering parameters (J-beam)

	trajectory	Side	number of measurement points
	$\log (C_{sa})$ 1		21
	$\log (D_a)$ 1	Int	21
Ī	$\log (C_{sa})$ 1	Ext	16
	$\log (D_a) 2$	Ext	16

Table 7- Geostatistical pre-study of the trajectories of the logarithms of the average diffusion parameters – *J-beam*

		F	Schii-variogranis and mst	analysis of experimental semi-variograms and histograms										
chgpt(*)	nugget	oscillations	original tangent	histogram shape	average	field	covariance							
0	no	no	insufficient resolution	asymmetrical	cste	GRF	Gauss or expon							
0	no	no	insufficient resolution	symmetrical	cste	GRF	Gauss or expon							
-	-	-	-	-	cste	GRF	Gauss or expon							
-	-	-	-	-	cste	GRF	Gauss or expon							
	0 0 - - s necessary	0 no	0 no no	0 no no insufficient resolution	0 no no insufficient resolution asymmetrical	0 no no insufficient resolution asymmetrical cste 0 no no insufficient resolution symmetrical cste cste	0 no no insufficient resolution asymmetrical cste GRF 0 no no insufficient resolution symmetrical cste GRF cste GRF							

Table 8- Geostatistical parameter estimates of the trajectories of the logarithms of the mean scattering parameters by SCAP-1D

	model					Σ_{exp} -tests				Σ_{mod} -tests			estimates			CoV			model selection		range		
traj	#	model	cov	chg	reg	χ²	KS	KPSS	DF	χ^2	KS	KPSS	DF	α1	s	a1 (cm)	α1	s	a1	AICc	ER	r ₉₅ (cm)	r ₉₅ /L
log(C) 1	1	GRF	expon	0	0	0	0	0	1	0	0	0	1	-4,82	0,13	17	0,02	0,31	0,81	-14,08	2,13	50	0,05
$\log(C_{sa})$ 1	2	GRF	gauss	0	0	0	0	0	1	0	0	0	1	-4,81	0,15	37	0,02	0,39	0,33	-15,59	1,00	63	0,07
log(C) 2	1	GRF	expon	0	0	0	0	0	1	0	0	0	1	-5,20	0,07	19	0,01	0,36	1,20	-19,22	1,00	57	0,06
$\log(C_{sa})$ 2	2	GRF	gauss	0	0	0	0	0	1	0	0	0	1	-5,20	0,07	20	0,01	0,36	0,80	-19,11	1,06	35	0,04
log(D) 1	1	GRF	expon	0	0	0	0	0	1	0	0	0	1	-27,48	0,31	10	0,00	0,31	1,65	3,78	1,01	29	0,03
$log(D_a)$ 1	2	GRF	gauss	0	0	0	0	0	1	0	0	0	1	-27,48	0,31	18	0,00	0,31	0,77	3,76	1,00	32	0,03
$log(D_a)$ 2	1	GRF	expon	0	0	0	0	0	1	0	0	0	1	-27,56	0,29	13	0,01	0,35	1,40	4,19	1,00	40	0,04
$\log(D_a)$ 2	2	GRF	gauss	0	0	0	0	0	1	0	0	0	1	-27,56	0,29	19	0,01	0,35	0,72	4,21	1,01	33	0,04

849 **15. List of Figure Captions**

- 850 Figure 1 Illustration of the geostatistical formalism: GRF Y
- 851 Figure 2 Illustration of the geostatistical formalism: noisy GRF Z
- Figure 3 General flowchart of SCAP-1D
- Figure 4 Interpretability of practical range (Gaussian autocorrelograms $\rho(h)$ with three practical
- range values, measurement step $\Delta x = 5$
- Figure 5 Size and position of rebars of the "J-beam" (distance unit: cm; diameter unit: mm)
- Figure 6 Different views: a) Aerial view, b) Sketch of bridge 3, 4 and 5, c) Beam J, d) Exposed
- 857 (Ext) and sheltered (Int) side of beam J.
- Figure 7 Samples taken for chloride titration (J-beam): a) Locations, b) Cross-section, c) Slices
- Figure 8- Profiles of the evolution of the statistical mean and standard deviation of total chloride
- 860 concentrations according to the depth of embedding
- Figure 9 Total chloride concentration database and illustration of the principle of implementing
- 862 SCAP-1D on total chloride concentration data
- Figure 10 Trajectories and statistical averages of the logarithms of the mean diffusion parameters
- at 28 years (J-beam)
- Figure 11 Raw trajectories and associated statistical means trajectory number / depth
- 866 correspondences presented in Table 5
- Figure 12 Selected trajectories and associated statistical means
- Figure 13 Profiles of the evolution of the mean and standard deviation of total chloride content
- according to the depth model a
- 870 Figure 14 Study of the Signal to Noise Ratio model a
- Figure 15 Profiles of the evolution of the mean and standard deviation of total chloride
- 872 concentrations according to the depth of embedding model b
- 873 Figure 16 Practical range profiles of total chloride concentration trajectories as a function of
- 874 embedment depth model b

875	Figure 17- Data and averages of trajectory 1 of log mean surface concentration; histograms of
876	trajectories so centered; experimental semivariogams of trajectories so centered
877	Figure 18- Data and averages of trajectory 1 of logarithm of mean diffusivity; histograms of
878	trajectories so centered; experimental semivariogams of trajectories so centered
879	Figure 19- Data and averages of logarithm of mean surface concentration trajectory 2; histograms of
880	trajectories so centered; experimental semivariogams of trajectories so centered
881	Figure 20- Data and averages of trajectory 2 of logarithm of mean diffusivity; histograms of
882	trajectories so centered; experimental semivariogams of trajectories so centered
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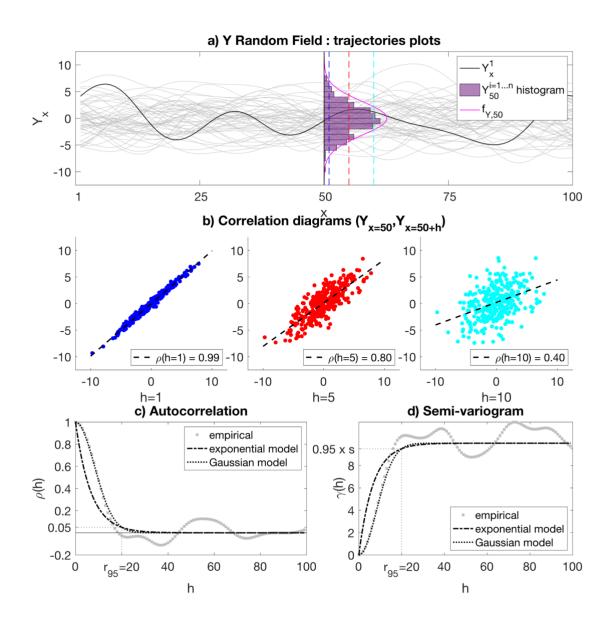


Figure 1 - Illustration of the geostatistical formalism: GRF Y

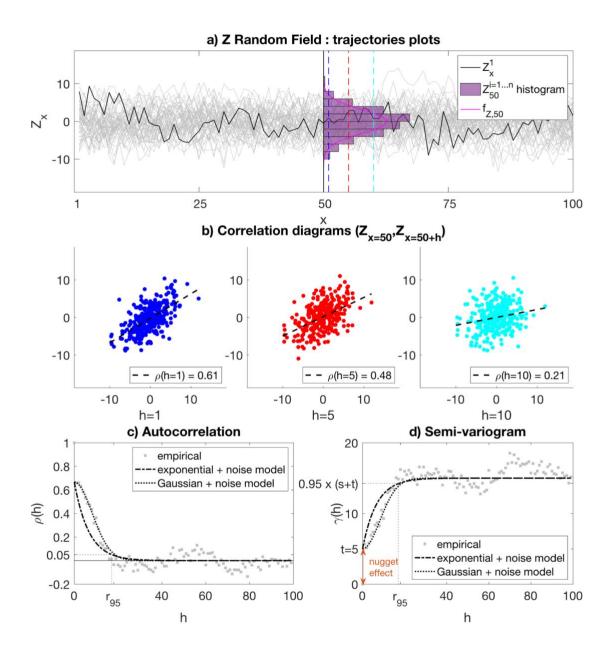


Figure 2 - Illustration of the geostatistical formalism: noisy GRF Z

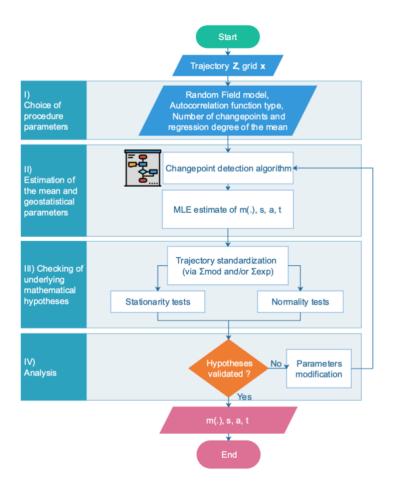


Figure 3 - General flowchart of SCAP-1D

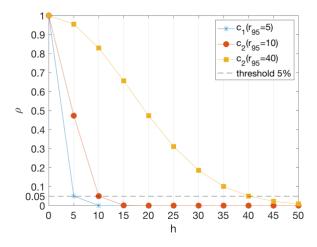


Figure 4 - Interpretability of practical range (Gaussian autocorrelograms $\rho(h)$ with three practical range values, measurement step $\Delta x = 5$)

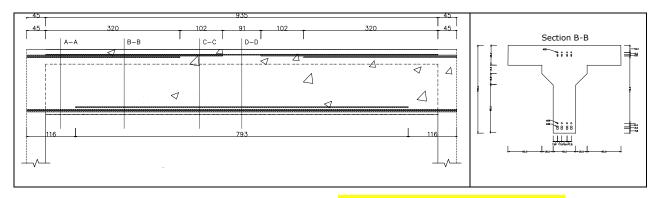


Figure 5 - Size and position of rebars of the "J-beam" (distance unit: cm; diameter unit: mm)

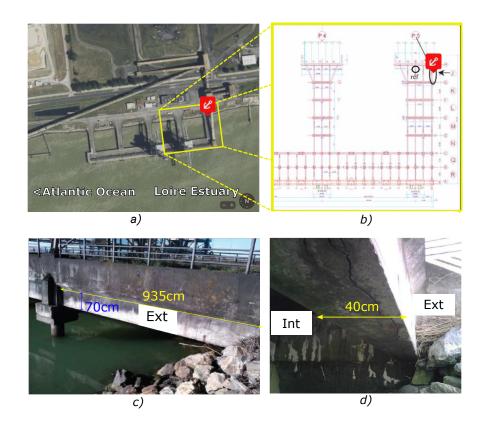


Figure 6 - Different views: a) Aerial view, b) Sketch of bridge 3, 4 and 5, c) Beam J, d) Exposed (Ext) and sheltered (Int) side of beam J.

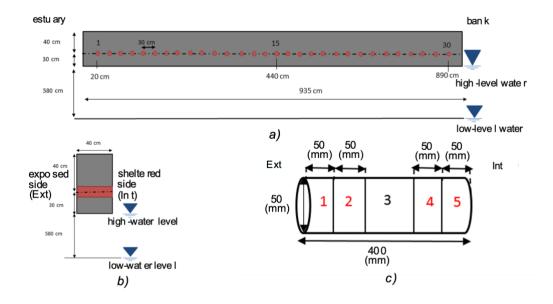


Figure 7 - Samples taken for chloride titration (J-beam): a) Locations, b) Cross-section, c) Slices

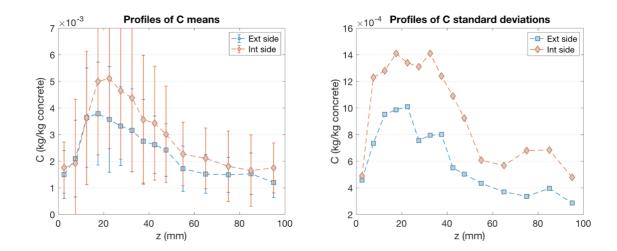


Figure 8- Profiles of the evolution of the statistical mean and standard deviation of total chloride concentrations according to the depth of embedding

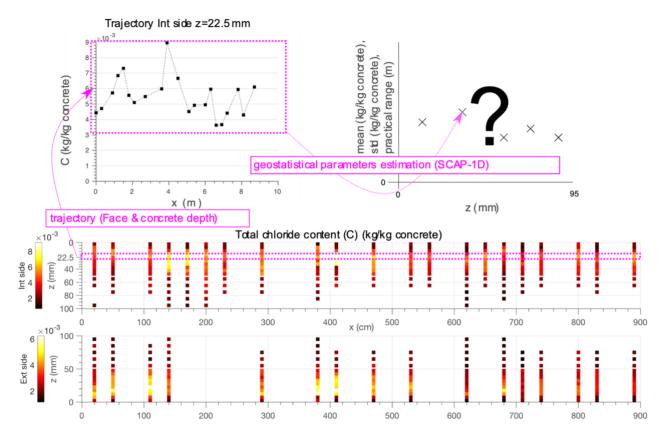


Figure 9 - Total chloride concentration database and illustration of the principle of implementing SCAP-1D on total chloride concentration data

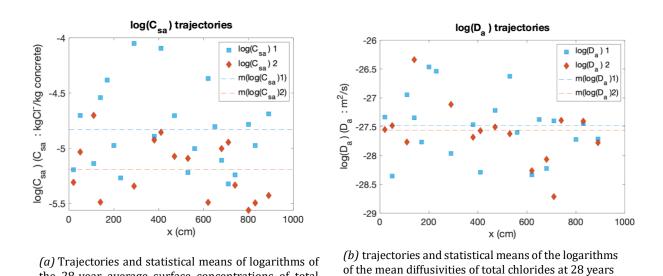


Figure 10 - Trajectories and statistical averages of the logarithms of the mean diffusion parameters at 28 years (J-beam)

- beam J

the 28-year average surface concentrations of total

chlorides - beam J

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909 910

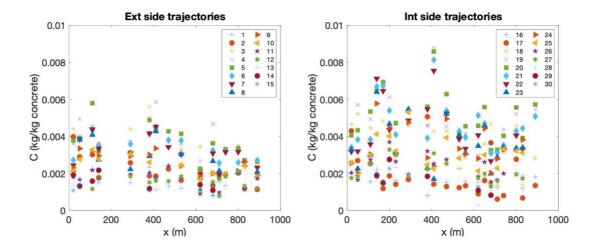


Figure 11 - Raw trajectories and associated statistical means - trajectory number / depth correspondences presented in Table 1

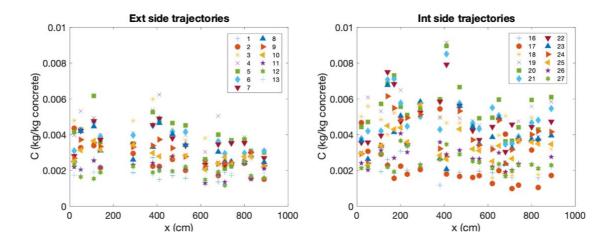
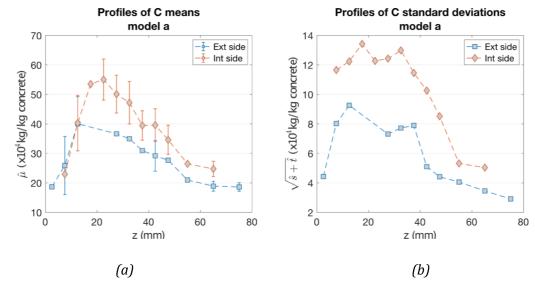


Figure 12 - Selected trajectories and associated statistical means



Figure~13-Profiles~of~the~evolution~of~the~mean~and~standard~deviation~of~total~chloride~content~according~to~the~depth~-model~a

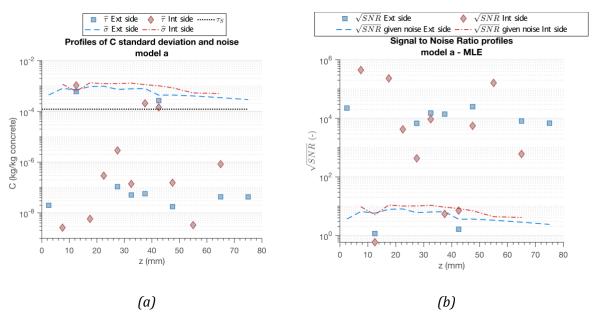
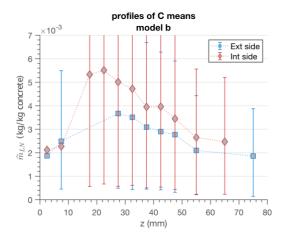


Figure 14 - Study of the Signal to Noise Ratio - model a



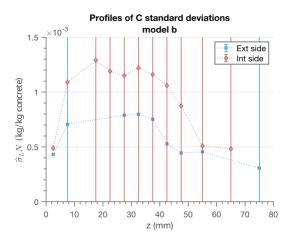


Figure 15 - Profiles of the evolution of the mean and standard deviation of total chloride concentrations according to the depth of embedding - model b

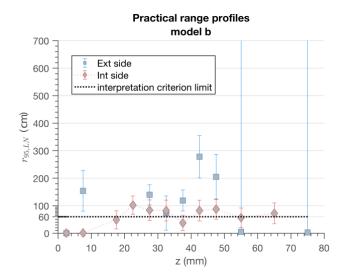


Figure 16 - Practical range profiles of total chloride concentration trajectories as a function of embedment depth - model b

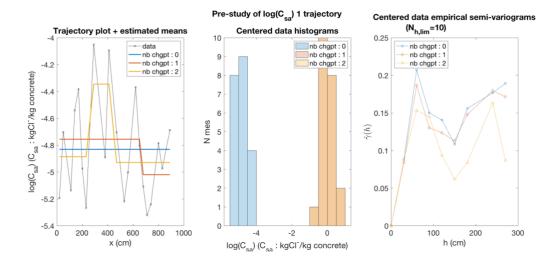


Figure 17- Data and averages of trajectory 1 of log mean surface concentration; histograms of trajectories so centered; experimental semivariogams of trajectories so centered

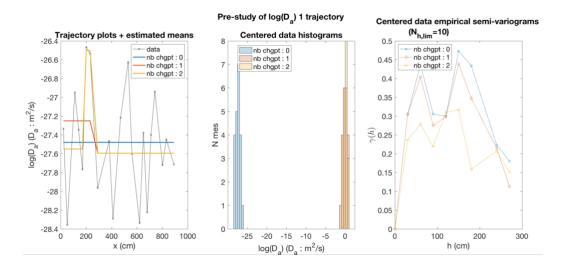


Figure 18- Data and averages of trajectory 1 of logarithm of mean diffusivity; histograms of trajectories so centered; experimental semivariogams of trajectories so centered

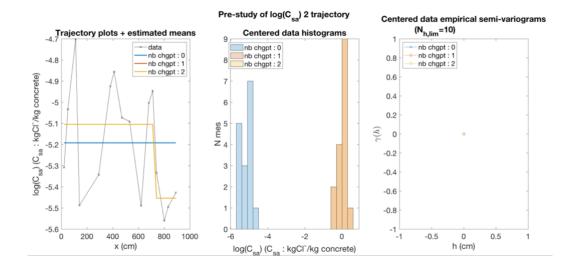


Figure 19- Data and averages of logarithm of mean surface concentration trajectory 2; histograms of trajectories so centered; experimental semivariogams of trajectories so centered

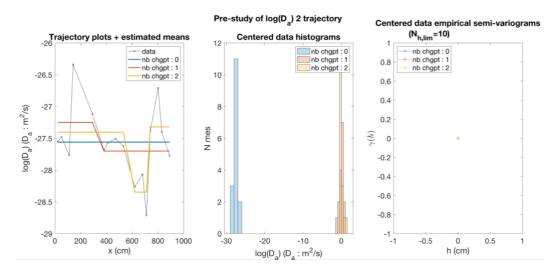


Figure 20- Data and averages of trajectory 2 of logarithm of mean diffusivity; histograms of trajectories so centered; experimental semivariogams of trajectories so centered