

### Modeling of chloride spatial variability in a reinforced concrete wharf from on-site measurements

Romain Clerc, Franck Schoefs, Mestapha Oumouni, Othmen Ines, Stéphanie

Bonnet

### ► To cite this version:

Romain Clerc, Franck Schoefs, Mestapha Oumouni, Othmen Ines, Stéphanie Bonnet. Modeling of chloride spatial variability in a reinforced concrete wharf from on-site measurements. ASCE-ASME Journal of Risk and Uncertainty in Engineering Systems, Part A: Civil Engineering, inPress, 10.1061/AJRUA6/RUENG-1214. hal-04494998v2

### HAL Id: hal-04494998 https://nantes-universite.hal.science/hal-04494998v2

Submitted on 24 May 2024

**HAL** is a multi-disciplinary open access archive for the deposit and dissemination of scientific research documents, whether they are published or not. The documents may come from teaching and research institutions in France or abroad, or from public or private research centers. L'archive ouverte pluridisciplinaire **HAL**, est destinée au dépôt et à la diffusion de documents scientifiques de niveau recherche, publiés ou non, émanant des établissements d'enseignement et de recherche français ou étrangers, des laboratoires publics ou privés.

1	Modeling of chloride spatial variability in a reinforced concrete wharf from on-
2	site measurements
3 4 5	Romain CLERC <sup>1</sup> , Franck SCHOEFS <sup>1</sup> , Mestapha OUMOUNI <sup>1</sup> , Inès OTHMEN <sup>2</sup> , Stéphanie BONNET <sup>2</sup>
6	<sup>1</sup> Nantes Université, École Centrale Nantes, CNRS, GeM, UMR 6183, F-44000 Nantes, France
7	<sup>2</sup> Nantes Université, École Centrale Nantes, CNRS, GeM, UMR 6183, F-44600 Saint-Nazaire,
8	France
9	* Corresponding author email: <u>franck.schoefs@univ-nantes.fr</u>
10	
11	
12	
13	

#### Abstract

15 Chloride ingress by diffusion is the major deterioration process of reinforced concrete (RC) structures exposed 16 to the marine environment. These structures have significant lengths or surfaces exposed to the outside 17 environment. Due to the material variability (different concrete batches, vibrations) and exposure variability, 18 the material experiences a spatial variability of the deterioration process. This paper presents the geostatistical 19 analysis of in-situ chloride profiles, leading to the assessment of the spatial variability (SV) of both the chloride 20 ingress itself and the parameters of the widely used Fick diffusion law (the average surface chloride content, 21  $C_{sa}$ , and the average chloride diffusion coefficient,  $D_a$ ). 37 chloride profiles measured on both sides of the 22 same spandrel beam of an RC wharf are studied, as well as the associated estimates of  $C_{sa}$  and  $D_a$ . From an 23 initial selection of random field models, the geostatistical analysis consists in the evaluation of model 24 parameters using a procedure that tests both data and model assumptions on the fly (ergodicity, stationarity, 25 random field modeling). Combined with the calculation of information criteria for each model, this procedure 26 allows to provide relevant geostatistical models for chloride ingress,  $C_{sa}$  and  $D_a$ , which render SV as well as 27 measurement error. It is noteworthy that the estimation error can be neglected when focusing on the SV for 28 the range of chloride content studied in this paper. The SV of the chloride content seems to depend on the 29 depth, with a large variability within the convection zone, and much less and more stable in the diffusion zone 30 with a practical range of about 70 cm. This order of magnitude is consistent with the range of SV calculated 31 for  $C_{sa}$  (50 to 73 cm).

32 Keywords: Chloride ingress, Reinforced concrete, Marine environment, Spatial variability,

33 Random field, Statistical analysis.

#### 34 **1. Introduction**

35 Chloride ingress is among the main causes of degradation of reinforced concrete (RC) structures 36 where the deterioration is more perceptible in marine environment. That leads to major maintenance 37 costs (Bastidas-Arteaga & Schoefs, 2015; Stewart & Val, 2003). These structures offer to the 38 environment important lengths or surfaces. Due to the variabilities of both the material (different 39 concrete batches, vibrations) and exposure, they experience a spatial variability of their deterioration 40 process. Accounting for realistic spatial correlation of the different parameters in structural safety 41 models may lead to more realistic estimates on structural reliability and maintenance planning, 42 knowing that reliability assessment is affected by spatial correlation (Schoefs et al., 2022; Stewart, 43 2004; Stewart & Mullard, 2007). Moreover, quantifying this property allows optimizing inspection

44 meshing on a structure (Gomez-Cardenas et al., 2015; Oumouni & Schoefs, 2021b; Schoefs et al.,
45 2017a).

46 The variability of the degradation parameters and their measurements can be described at47 different scales:

the point scale, characterizing the repeatability of measurements;
the local scale of the material, characterizing the heterogeneity of the material for a scale
lower than the Representative Elementary Volumes (REV) associated with each of the
quantities of interest (Hill, 1963);

higher scales (structural elements, structure), characterizing the degree of spatial
heterogeneity of the material and/or environmental conditions (exposure, loading, ...) for
scales above the REVs.

55 The restitution of point and local variabilities requires a statistical modeling, translating the hypothesis of independence, respectively on (i) the measurements of parameters between distinct 56 57 measurements and for a same measurement point and (ii) the parameter values between similar 58 structures for the same REV dimension. The spatial variability (taking into account local and larger 59 scales) cannot be solely captured by a statistical modelling because of the hypothesis of the existence 60 of a certain degree of heterogeneity, i.e. spatial correlation, between measurements or between 61 parameter values associated with different measurement points or REV on the same structure. This can only be translated by geostatistical modeling. The major challenge of those models is to determine 62 63 stochastic properties of the Random Fields (RF), namely their marginal distributions, stationarity and 64 the spatial correlations.

65 Chloride ingress has been investigated deeply since the 70's and it has been shown how 66 complex is the process physically and chemically (Glasser et al., 2008; Kaushik & Islam, 1995; Ragab 67 et al., 2016; Safhi et al., 2019; Wegian, 2010) and the role the material complexity (porosity) is

playing (Qu et al., 2021). For the moment, available models of transfer and knowledge don't allow 68 69 to represent accurately the spatial variability of pollutant in concrete (Ravahatra et al., 2017). That is 70 the main reason why direct or indirect assessment of chlorides in situ is the only way to measure, 71 analyze and model the spatial variability of chloride ingress. Then, two kinds of quantities of interest 72 can be considered:

- 73 the total chloride content *C* at several depths;
- 74

their mean diffusion parameters  $C_{sa}$  and  $D_a$ , estimated from profiles of chloride content. •

75 During the last decades sensors have experiences significant progresses (Biondi et al., 2021; Torres-Luque et al., 2014, 2017; Watanabe et al., 2021; Zaki et al., 2015) but there are few concepts 76 77 that measure the spatial distribution of chloride ingress (Fares et al., 2018; Lecieux et al., 2015, 2019) 78 and their implementation is rare (Priou et al., 2019) even if their interest in terms of the value of 79 information they provide was shown (Schoefs et al., 2022) especially because they allow to filter the 80 signal (Oumouni & Schoefs, 2021a). Moreover, these sensors have to be installed at the building 81 stage and are not suitable for existing structures.

82 That is the main reason why spatial variability is assessed through Non-Destructive Testing 83 (NDT) tools or Semi-Destructive Techniques (SDT). On-site indirect assessment of chlorides through 84 NDT-techniques is affected by errors of assessment and sensitivity to other parameters (humidity, 85 temperature) which makes it difficult to assess the spatial variability of chlorides (Bonnet et al., 2017; 86 Bourreau et al., 2019; Schoefs et al., 2023). This error affects the optimization of measurements for 87 assessing spatial variability (Oumouni & Schoefs, 2021b). That is why, the rare studies are based on 88 the semi destructive techniques (SDT) i.e. direct weighting of chlorides from cores (O'Connor & 89 Kenshel, 2013; Othmen et al., 2018) reviewed 10 published studies from 2004 to 2016 with only 3 90 concerning more than 30 measurements and in all cases the distance between measurement was too 91 large in view to assess the spatial correlation. Two main quantities of interest are investigated: 92 chloride content at a given depth and parameters of a model of chloride ingress after post-processing 93 of chloride profiles.

94 Because of the complexity and cost for making a large amount of cores in a beam, only a few 95 amounts of data is available for each and the distance between cores is too large: 5 cores with a 96 distance of about 1 m in the study of (O'Connor & Kenshel, 2013). That leads to a significant 97 numerical error of assessment of the spatial correlation (Schoefs et al., 2017b). Moreover, the 98 underlying assumptions for RF modelling that are stationarity and ergodicity are not checked. Finally, 99 the error of measurement is usually not considered when identifying correlations. That is a drawback 100 because it is well known that SDT measurements for existing structures (Schoefs et al., 2023) and 101 even for concrete in laboratory (Bonnet et al., 2017; Hunkeler et al., 2000) are affected by a significant 102 error. To overcome these limits, this paper relies on two pillars: first, a unique database and second, 103 a geostatistical analysis that allows to provide geostatistical parameters (namely spatial correlation 104 assessments as well as measurement error) by using a recent method of RF identification (SCAP1D, 105 Clerc et al. 2019), that accounts for uncertainty of measurements, checks ergodicity and accounts for 106 the non-stationarity of the RF.

107 Concerning the first pillar, marine structures can be exposed in various environments: 108 atmospheric, spatter or splash, tidal and underwater (BS 6349-1:2000, 2000) which were defined 109 according to the tide range (Bourreau et al., 2020). In this work, 30 cores carried out every 30 cm on 110 the same 28-year beam (Othmen et al., 2018) are analyzed. The beam is located in the worst 111 environment that is splash zone (Angst et al., 2009), and the cores are through its width giving access 112 to profiles from each side i.e., exposure conditions (exposed, sheltered).

113 Concerning the second pillar, two objectives are targeted: the assessment of spatial variability 114 of the total chloride content *C*, depending on depth and exposure on one hand and the assessment of 115 the spatial variability of the mean diffusion parameters  $C_{sa}$  and  $D_a$  on the second one. First, taking 116 benefit of the availability of *C* at several depth, the objective is to provide estimates of its geostatistical 117 parameters and to answer the following questions: (i) Is the spatial variability (SV) of *C* a function of 118 depth? (ii) Is the SV of *C* a function of the exposure conditions? Second, knowing the age of the 119 structure (28-year), geostatistical parameters of mean diffusion parameters are estimated to answer 120 the following question: Is the SV of  $C_{sa}$  and  $D_a$  a function of the exposure conditions? The latter 121 question arises because  $C_{sa}$  and  $D_a$  are directly dependent on the chloride profiles, which vary 122 markedly between the exposed and sheltered faces.

123 Section 2 introduces the geostatistical formalism in a theoretical way before presenting a state 124 of the art of RF modeling, simulation methods and spatial variability estimation methods. An attention 125 is paid to the underlying assumptions of the estimation methods and to the consequences of their neglect on the estimated spatial variability, especially in the frequent case of non-stationary 126 127 measurements due to spatially variable exposure conditions. Section 3 presents the structure and the 128 available data. Sections 4 and 5 give the results at two levels: total chloride contents, and mean 129 diffusion parameters of total chlorides. Section 6 concludes the paper with some extension to other 130 structures or other exposures.

#### 131 2. Geostatistical methods for spatial variability assessment of concrete structures

#### 132 2.1. Modelling of spatial variability

#### 133 2.1.1. Random Field modeling

Practical modeling of spatial variability implies modeling spatially variable quantities of interest (QI) as Second-Order Intrinsic Stationary Random Fields (RF) (Chilès & Delfiner, 2012). Following the geostatistical formalism, such an RF *Z* is defined on a domain  $D^n$  of  $\mathbb{R}^n$  and on a probability space ( $\Omega, A, P$ ) such that:

138 •  $\forall x \in D^n, Z(x, \cdot) = Z_x$  is a multivariate random variable of  $\omega \in \Omega$  with a joint probability density

139 function (PDF)  $f_Z(\boldsymbol{m}, \boldsymbol{\Sigma})$ , where  $\boldsymbol{m}$  and  $\boldsymbol{\Sigma}$  are the mean vector and the covariance matrix of  $Z_x$ ,

140 respectively;

141 •  $\forall x \in D^n$  and  $\forall \omega \in \Omega$ ,  $Z(x, \omega) = Z_x^{\omega}$  is a realization of Z georeferenced by the vector x, also 142 called a *trajectory* (in practice, a set of measurements from a unique structure and at a unique 143 time).

144 •  $\forall (h, x) \in D^2$ ,  $\mathbb{E}[Z_{x+h} - Z_x] = 0$  and  $Var(Z_{x+h} - Z_x) = 2\gamma(h)$  where  $\gamma(\cdot)$  is the *semi-*145 *variogram* function of Z and *h* is any *distance-lag* (distance between two points of D).

146 This definition reflects both the variability of the input parameter between structures and its spatial 147 variability within the same structure, which are embedded in the *covariance matrix*  $\Sigma$  and the semi-148 variogram.

149 In this paper, data are modeled as (i) unidimensional (ii) noisy (iii) trend-stationary (iv) Gaussian 150 Random Field (GRF). Indeed, (i) the data are unidimensional along the sides of the beam; (ii) if the 151 spacing between measurements is greater than the REV, the RF model must be able to reproduce the 152 point and local variabilities, which have no correlation structure and can be modeled by white noise; 153 (iii) materials and environmental conditions may vary along the sides of the beam. It is then very likely that the mean of the quantities of interest will vary with space; (iv) GRFs are very convenient 154 because they allow modeling most natural variabilities (following normal or lognormal marginal 155 156 laws) with only a model for the mean and the covariance (Chilès & Delfiner, 2012).

157 Mathematically,  $\forall x \in D^n$  we thus consider Z such that

$$Z_x = m_x + W_x + Y_x \tag{1}$$

(2)

158 with  $m_x$  a deterministic trend,  $W_x \sim N(\mathbf{0}, t \times \mathbf{1}_n)$  a white noise of variance t;  $Y_x \sim N(\mathbf{0}, \boldsymbol{\Sigma}_Y)$  a 159 centered GRF, i.e  $Y_x$  a multivariate normal variable such that

$$\forall (h, x) \in D^2, \Sigma_{Y_{x+h}} \equiv Cov(Y_{x+h}, Y_x) \equiv C_Y(h) \equiv s \times \rho_Y(h)$$

160 where  $C_Y(\cdot)$  is the covariance function of *Y*, *s* is its variance,  $\rho_Y(\cdot)$  is its autocorrelation function, and 161 *h* is any distance-lag between the data (equivalences due to the Second-Order Stationarity of the 162 GRFs). 163 This formalism is illustrated in Figure 1 and Figure 2, where trajectories, correlation plots, 164 autocorrelation functions, and semivariograms are plotted for *Y* and *Z* respectively, defined such that:

165 1) 
$$Y_x \sim N(\mathbf{0}, \boldsymbol{\Sigma}_Y)$$
 with  $\Sigma_{Y_{x+h,x}} = s \times \exp(-1.73 \|h\|^2 / r_{95}), \ \rho_Y(h) = \frac{2\gamma_{x+h,x}}{s}, \ \gamma_Y(h) = s - \frac{2\gamma_{x+h,x}}{s}$ 

166 
$$\Sigma_{Y_{x+h,x}}, s = 10 \text{ and } r_{95} = 20;$$

167 2) 
$$Z = W + Y$$
 with  $W_x \sim N(\mathbf{0}, t \times \mathbf{1}_n)$ ,  $\Sigma_{Z_{x+h,x}} = t \times \delta_{x+h,x} + \Sigma_{Y_{x+h,x}}$ ,  $\rho_z(h) = \frac{\Sigma_{Z_{x+h,x}}}{s+t}$ ,  
168  $\gamma_Z(h) = s + t - \Sigma_{Z_{x+h,x}}$ , and  $t = 5$  ( $\delta_{ij}$  is the Kronecker delta).

169 2.1.2. Autocorrelation models and parameters

- 170
- 171

#### 172 **3. Embedded Tables**

173 Table 5 and Figure 1 present and illustrate the exponential and Gaussian autocorrelation functions,

- 174 widely used in the literature. According to Equation (2), such functions are positive definite, so that
- 175 they asymptotically decay as a function of the distance-lag h. The evolution of this decay is mainly
- 176 determined by the value of their scale parameter a > 0, which parametrizes the extent of the spatial
- 177 correlation, i.e. the distance threshold below which the QI are strongly correlated.
- 178 The distance at which events are weakly correlated is referred to as the inspection distance threshold
- 179 with a spatial correlation threshold definition of 30% in (Schoefs et al., 2016). Several notations and
- 180 parameters coexist in the literature to describe this threshold, sometimes with some confusion. In the
- 181 case of isotropic fields, the following three notations are commonly used: (i) Cressie (1993, p. 118)
- 182 proposed the fluctuation range  $\theta$ , which is defined as  $\theta = \int_{-\infty}^{+\infty} \rho(h) dh$ ; (ii) (Cressie, 1993, p. 88)
- 183 proposed the practical range  $r_{95}$ , which is defined as  $\rho(r_{95}) = 5\%$  (see Figure 1 and Figure 2); (iii)
- 184 many authors proposed the correlation length  $l_c$ , which is not strictly defined and alternatively taken
- 185 to be equal to  $a, \theta, r_{95}$  or some other particular range. In order to avoid any confusion and to facilitate
- 186 comparison and interpretation, we give in

#### 188 **4. Embedded Tables**

Table 5 the expression of  $\theta$  and  $r_{95}$  as a function of *a* for the exponential and Gaussian models and the corresponding autocorrelation values  $\rho$ . Note that in Figure 1 *Y* is obtained from a Gaussian autocorrelation function.

#### 192 4.1. Identification of spatial variability from measurements

#### 193 4.1.1. Empirical identification

194 RF models behind a trajectory can be identified in the case of its *ergodicity*, which can be seen as a 195 mixture of stationarity and asymptotic independence. In this case, the spatial variability can be 196 assessed from empirical representations of the semivariogram, the covariance function, or the 197 autocorrelation function, called the empirical semivariogram, covariogram and autocorrelogram, 198 respectively.

The Semivariogram, denoted  $\hat{\gamma}(h)$ , is used because its estimation does not require complementary ones (such as mean or variance), and it allows to visually identify (i) the Second-Order Stationarity property, (ii) the ergodicity property, and (iii) the presence and magnitude of local variabilities. Indeed, (i)  $\hat{\gamma}(h)$  asymptotically tends to the variance for Second-Order Stationary RF, while otherwise whereas it increases with *h*; (ii) ergodicity is verified when  $\hat{\gamma}(h)$  reaches an asymptote in the limit of half of the domain *D* (Cressie, 1993, p. 57); (iii) white noise is easily identified by a *nugget effect*, i.e. a non-zero value for h = 0 (see Figure 2).

A common estimator of  $\hat{\gamma}(h)$  is defined by Equation (3) for a given trajectory  $Z_x^{\omega}$ , where  $S_h$  is the set

207 of pairs  $(x_p, x_q)$  of  $D^2$  that are away from h modulo a given tolerance, and  $N_h$  is the cardinal of  $S_h$ .

$$\hat{\gamma}(h) = \frac{1}{2N_h} \sum_{(x_p, x_q) \in S_h} \left( Z_{x_p}^{\omega} - Z_{x_q}^{\omega} \right)^2$$
(3)

We point out that when *h* becomes close to the dimension of *D*, statistical inference is no longer valid because the number of point pairs away from *h* is very small. Therefore, geostatisticians recommend plotting this graph only for distances *h* such that  $N_h \ge 30$  (Cressie, 1993). In practice,

- 211 in civil engineering, several authors recommend limiting to values of h less than half of the size of
- the domain (Arnaud & Emery, 2000, Schoefs et al., 2016).
- 213 *4.1.2. Estimates of model parameters*
- 214 Once an RF model is selected from empirical identification, its parameters (collected in a vector **p**)
- 215 must be estimated. Several methods are available:
- Maximum Likelihood Estimation (MLE), which allows to estimate *p* from raw data, given an RF
   marginal distribution model, and as the following properties: (i) asymptotically unbiased, (ii)
   consistent, (iii) equivariant, (iv) asymptotically efficient, and (v) asymptotically normal
   (Wasserman, 2004, para. 9.4);
- Least-Squares Estimate (LSE), which allows estimating *p* from semivariogram, covariogram or
   autocorrelogram given models of them;
- Weighted LSE (WLSE), similar to LSE, where each of the residuals is weighted by its variance.
- 223 This allows to increase the weight of the residuals computed from many data (low distance-lag h224 values, for which  $N_h$  is larger).
- 225 Usually, LSE and WLSE fit the covariograms well and the use of WLSE implies a better description
- of them for small distances, associated with more data<sup>1</sup>. However, covariograms are not necessarily
- 227 representative of the effective covariance parameters of the field, which can lead to significant relative
- 228 errors in their estimation. Because it considers only raw data, MLE provides more accurate estimates,
- with smaller relative errors. The trade-off is computational time, as MLE is more time consuming
- than WLSE and LSE, up to 10 times faster. Therefore, and because it is *a priori* complex to have an
- 231 initial idea of the scale parameter, a two-phase estimation method of Second-Order Stationary RF
- 232 parameters is preferred:

<sup>&</sup>lt;sup>1</sup> In this case,  $\gamma(h) = 1 - C(h)$ , so estimates on the covariogram or the semivariogram are equivalent.

- WLSE pre-estimation of the parameters with the following initial values: (i) for *s*, the empirical variance; (ii) for *a*, a value such that the fluctuation scale is less than a quarter of the maximum lag-distance (so as to respect the ergodic assumption);
- 236 2. MLE estimation of the parameters using the Nelder-Mead simplex minimization algorithm with
   the WLSE estimates as initial values.
- This approach allows to obtain accurate estimates while minimizing the cost of the MLE due to the definition of initial values close to the real values of the parameters (Clerc et al., 2019, para. 4.4).
- 240 4.1.3. Choice of best RF model

Although the plot of the empirical semi-variogram allows the selection of an RF model, its estimator nature leads to an imperfect representation of the SV. This is all the more true when the number of measurements is small. Therefore, when there is insufficient scientific evidence to support a specific RF model for a QI, which is the case for chloride content and diffusion parameters (Gomez-Cardenas et al., 2015), we can rely on data-based methods for model selection.

- 246 In the following, the Akaike Information Criterion (AIC) is preferred because it is much more robust
- 247 than the  $R^2$  coefficient of determination (Burnham & Anderson, 2010). The AIC allows to compare,
- 248 with the parsimony criterion, the relevance of any RF model  $M_Z$  with K parameters  $p_{1,\dots,K}$  to describe
- 249 the hidden RF Z at the origin of a set of n data  $Z_x^{\omega}$  (e.g., a trajectory). This criterion assumes that the
- 250 best model is the one that provides the most accurate description with the fewest possible parameters
- 251 (Burnham & Anderson, 2010, para. 1.4).
- In particular, when n/K < 40 and (which is the case below) we use the corrected information criterion (AICC). When the **p** estimate  $\hat{p}$  is computed with MLE, it writes:

254 
$$AIC_{c} = \underbrace{-2\ln(L(\hat{p}|Z_{x}^{\omega})) + 2K}_{AIC} + \underbrace{\frac{2K(K+1)}{n-K-1}}_{\text{correction}}$$

where  $L(\hat{p}|Z_x^{\omega})$  is the likelihood of  $\hat{p}$  with respect to the data. The best model among a set of considered models is then the one that minimizes the AICC, i.e. the one that has both a high goodness of fit (high likelihood) and a limited complexity (limited number of parameters). In practice, the easiest way to compare the relevance of models based on their AICC is via the Evidence Ratio (ER) (Burnham & Anderson, 2010, para. 2.10). This is defined such that, for any model  $M_{Z,i}$  among  $N_m$  models,

261 
$$ER_{i} = \exp\left(\frac{1}{2}\left[AICc_{i} - \min_{\substack{j=1,\dots,N_{m} \\ \text{best a priori model}}}\right]\right)$$

The ER is therefore 1 for the best *a priori* model and grows exponentially as the models become less fit. From an expert perspective, an ER of 2 for a given model does not justify preferring the best a priori model over it, whereas an ER of 18 clearly favors the best a priori model (Burnham & Anderson, 2010, para. 2.10).

#### 266 *4.1.4.* SCAP1D: a robust estimation procedure

267 The methods presented assume an adequate RF model to avoid estimation errors: without modeling 268 precautions, they are only valid under the condition that the data can be described by an SSO and 269 ergodic RF. However, since classical regression techniques coupled with generic autocorrelation 270 models can easily approximate autocorrelograms and covariograms, the estimation of RF model 271 parameters is typically performed by LSE on raw data without prior verification of these two 272 hypotheses (O'Connor & Kenshel, 2013; Ravahatra et al., 2017; Schoefs et al., 2016). Such a 273 procedure can then lead to significant errors, in especially in the estimation of the scale parameter a, 274 and especially in cases of non-stationarity in the mean and/or significant measurement noise. This 275 point is demonstrated in a case study by (Clerc, 2021, p. 143): from a given realization of a standard 276 GRF, adding a noise and/or a mean trend and performing an LSE of a without prior verification of the SSO and ergodic hypotheses led to estimation errors of up to 994%. 277

- To avoid these errors, we use here the SCAP-1D procedure<sup>2</sup> (Clerc et al., 2019). Based on previously
- presented tools, it allows to estimate, from a single trajectory with few measurement points, the

<sup>&</sup>lt;sup>2</sup> Spatial Correlation Assessment Procedure for Unidimensional Data

parameters of a unidimensional noisy trend-stationary RF with piecewise polynomial mean, constant
 variance and constant autocorrelation model.

The concept of SCAP-1D is to ensure the reliability of the parameters' estimates by testing and validating the following hypotheses:

- *H1*: validity of the RF mean model and its parameter estimates, i.e. Second-Order Stationarity of
   the centered RF;
- *H2*: validity of both the joint PDF and autocorrelation function models of the RF and of the
   estimation of their parameters; i.e., normality of the standard source GRF, obtained by
   isoprobabilistic transformation of the RF from estimates considering the *a priori* model;
- *H3*: ergodicity of the trajectory; i.e., non-correlation of the data at infinity or determination of all
   geostatistical parameters from this trajectory.
- The SCAP-1D flowchart is shown in Figure 3. It is divided into four consecutive steps, which can be repeated if the estimates do not validate the hypotheses on the RF model:
- Step 1) Choice of SCAP-1D parameters, namely piecewise polynomial mean model (regression degree, number of changepoints), RF joint PDF and autocorrelation function. We recommend to plot the trajectory, its histogram and its experimental semivariogram to facilitate this step.
- Step II) Estimation of the mean and geostatistical parameters: changepoints in the mean and their
   significance are determined using the PELT (Pruned Exact Linear Time) optimal partitioning
   algorithm of (Killick et al., 2012). Estimation of the RF parameters is then performed using the
   approach presented in 4.1.2<sup>3</sup>.
- Step III) Checking of the underlying mathematical hypotheses: the plausibility of the estimated
   model is verified by checking ergodicity and performing stationarity tests (DF-test (Dickey &
   Fuller, 1981), KPSS-test (Kwiatkowski et al., 1992)) and normality tests (χ2, KS-test
   (Kolmogorov-Smirnov)) on the standardized trajectory, based on the Step II estimates.

 $<sup>^{3}</sup>$  or by WLSE on the experimental semivariogram in the case of non-explicit density models.

304 • <u>Step IV</u>) <u>Analysis</u>: based on the Step III tests, the model selected in Step I and the estimates made
 305 in Step II are either accepted (algorithm stops) or rejected (modification of Step I parameters is
 306 required).

In Step III, the standardization of the trajectory is done by the Cholesky transformation, taking into
 account either:

- the empirical covariance matrix: in this case, we only test H1, since the tests results depend only on the data and the estimates of the mean. The tests are then called  $\Sigma_{exp}$ -tests;
- or the modeled covariance matrix: in this case, we test H1 to H3 together, since the tests results
- 312 depend on the total estimates. The tests are then called  $\Sigma_{mod}$ -tests.

#### 313 4.1.5. Interpretability criterion

In Step IV of SCAP-1D, the estimates must be analyzed for their physical consistency. In particular, an interpretability criterion can be defined for the estimated practical range. The point is to ensure, with respect to the distance  $\Delta x$  between measurements, that the variability thus described is indeed a spatial variability and not a local or point variability. This is illustrated in Figure 4, where the Gaussian autocorrelogram is plotted on a measurement grid of step  $\Delta x = 5$  for three practical range values ( $r_{95} < 2\Delta x$ ,  $r_{95} \simeq \Delta x$ , and  $r_{95} > 2\Delta x$ ):

• when  $r_{95} \ge 2\Delta x$  (curves c3 and c2): the variability described is spatial variability, since there is at least one significant intermediate spatial correlation value between the total correlation of a point with itself and the minimum 5% correlation associated with the practical range;

• when  $r_{95} < 2\Delta x$  (curve c1): the variability described may be local or point variability since there

is no significant intermediate spatial correlation value to account for the spatial variability.

325 Therefore, care will also be taken to ensure that the criterion  $r_{95} \ge 2\Delta x$  is met when applying SCAP-326 1D<sup>4</sup>.

#### **5.** Presentation of the structure and the measurements

#### 328 5.1. Presentation of the wharf, the beam and its material

329 The reinforced concrete beam inspected within the framework of the APOS research project is a T hyperstatic beam called "J-beam", whose dimensions and reinforcement are shown in Figure 5. It is 330 331 located on lane 51 of the gangway 5 of the Montoir-de-Bretagne coal terminal (Loire-Atlantique 332 department, France) with GPS coordinates: 47.32°N, -2.17°E. This terminal, built between 1981 and 333 1983 on the banks of the Loire and less than 7 km from the Atlantic Ocean, is 234.4 m long and 20.7 334 m wide and is located at +8.40 m NGF (Figure 6). This proximity to the ocean implies the existence 335 of a tidal phenomenon with an amplitude of 5.80 m and a salinity of the water corresponding to 336 maritime conditions at high tide (about 30 g Cl<sup>-</sup>/L). Thus, at the highest tidal coefficients, the lower 337 face of beam J, located 5.80 m above the level of the Loire, is in contact with the water for about 2 338 days per month. Finally, one of the peculiarities of the J-beam is its location at the edge of the quay. 339 It has a side exposed to the wetting-drying cycles, marked Ext, and a sheltered side, marked Int (see 340 Figure 6).

341

Data from the construction archives indicate that the concrete used to build the structure is a Portland cement concrete CEMI 42.5N dosed at 350 kg/m3. The mass fraction of  $m_{cl}$  clinker is therefore higher than 90% and that of secondary constituents such as gypsum does not exceed 5%. The aggregates used are sand 0/6, and gravel 5/10 and 10/25. Concrete water porosity  $\varepsilon$ , measured on the central part of 5 specimens used for chloride titration, was 13.7% on average, with minimum and

<sup>&</sup>lt;sup>4</sup> This criterion is constructed similarly to that of (Der Kiureghian & Ke, 1987) on the ability of a discretization grid to reproduce the spatial variability properties of an RF.

maximum values of 11.3% and 15.9%, respectively. The average compressive strength  $f_{cm}$ , measured on 4 specimens of 5 cm diameter and 10 cm length, was 43.5 MPa, with minimum and maximum values of 38.5 MPa and 48.9 MPa, respectively.

#### 350 5.2. Total chloride content measurements at 28 years

#### 351 5.2.1. Available data set

352 Chloride content measurements were performed on 30 through cylindrical cores extracted on the Jbeam at the same height of 30 cm and with a lateral spacing of 30 cm (see Figure 7) (Othmen et al., 353 354 2018). The extraction was performed in 2011-2012, after 28 years of exposure. From initial dimensions of 50 mm in diameter and 400 mm in length, each core was divided into 5 slices 355 356 (identified in Figure 7). Slices 1, 2, 4 and 5 were used to determine the chloride profiles according to 357 the procedure recommended by the RILEM TC 178-TMC (Vennesland et al., 2013). Slices 1 and 2 are related to the outer side (collected in Ext specimens) and slices 4 and 5 are related to the inner 358 359 side (collected in Int specimens). While deep slices 2 and 4 were 40 mm thick with measurements in 10 mm increments, slices 1 and 2 were 45 mm thick with measurements in 5 mm increments to 360 capture the value at the surface. Slice 3 was used for further investigations such as porosity, 361 362 compressive strength and estimation of the initial chloride content. Out of a total of 30 samples per side, 16 were usable on the Ext Face (53%), and 21 on the Int Face (70%). Furthermore, of the 555 363 total chloride contents of the 30 samples considered, only 460 were usable (83%, or 51% of the total 364 originally planned measurements). 365

366

Figure 9 hereafter then represents the remaining set of total chloride measurements at 28 years finally
considered (marked *C* in the following). The evolution of the empirical mean and standard deviation
by depth and by face is shown in Figure 8.

#### 370 5.2.2. Constitution of trajectories

In the following, the evolution of the SV of *C* is analyzed as a function of depth and exposure conditions. SCAP-1D is then applied to horizontal trajectories of measurements made on the same face and at the same embedding depth *z*. The estimation of geostatistical parameters for each trajectory then allows plotting their evolutions as functions of *z* and exposure face. This study principle is illustrated in Figure 9.

#### 376 5.3. Average diffusion parameters at 28 years

The 37 total chloride profiles were post-processed to determine the associated Fick's second law average diffusion parameters at the reference time  $t_r$  of 28 years, denoted  $C_{sa}$  and  $D_a$ . The fitting procedure to the peak of the profiles is described in (Othmen et al., 2018) and implemented in (Schoefs et al., 2023). Thus, one trajectory for each diffusion parameter per side is constructed at a height of 30 cm. These include 16 points for the Ext Side and 21 points for the Int Side, with a minimum distance of 30 cm between measurement points.

Table 6 summarizes this information and Figure 10 shows the trajectories. Note that the log function is applied to each parameter to (i) simplify the representation and visual analysis as they are assumed to be lognormally distributed, (ii) use identification procedures (see section 4.2) available for normally distributed random fields.

# 387 6. Spatial variability estimates of total chloride content in J-beam and physical 388 understanding

#### 389 6.1. Preamble: trajectories conditioning (kriging, debiasing)

Taking into account the non-usable or missing measurements per sample and in order to take into account the maximum number of points per trajectory, these are first enriched by ordinary kriging of the available data (Baillargeon, 2005, Chapter 4; Chilès & Delfiner, 2012, Chapter 3). The preliminary plot of the raw trajectories does not show any obvious non-stationarity (Figure 11), and the expectation of the associated RF is not known. 395 The kriging is then performed from the Weighted Least-Square Estimate (WLSE) identification on 396 the semivariograms of the initial trajectories. We consider an exponential covariance model and the 397 presence of an additive measurement error, identified by (Bonnet et al., 2017). In addition, (Bonnet et al., 2017) also identified a measurement bias  $b = 0.37 \times 10^{-3}$  (kg/kg concrete) associated with the 398 399 measurement protocol used and with human errors during its implementation. All the measurements 400 are therefore debiased before kriging so that the data considered are now C + b. In order to limit the 401 influence of the kriged data on the estimation of the SV of C, only the trajectories for which their proportion is less than 25% are considered. 402

403 In total, 13 trajectories with 16 points can be considered on the Ext side ( $z \in [2.5; 75]$  mm) and 12 404 trajectories with 21 points on the Int side ( $z \in [2.5; 65]$  mm). The number of kriged data per trajectory 405 is detailed in Table 1 in the Appendix, and the selected debiased trajectories are shown in Figure 12. 406 Less than 25% of the data are kriged to 65 mm depth for both sides. We point out that due to the 407 absence of measurements on the abscissae of the non-exploitable specimens, the trajectory grids are 408 irregular, with 30% of measurements missing for the Int side and 53% for the Ext side. The 409 performance of the SCAP-1D normality tests is therefore affected (Clerc et al., 2019), and special 410 care must be taken to ensure the consistency of the estimation results.

#### 411 6.2. Geostatistical pre-study and choice of parameters (Step I):

412 We recall that the studied trajectories are debiased. We then denote  $\tilde{Z}$  the RF of C such that

$$\tilde{Z} = Z - b \qquad | \qquad b = -0.37 \times 10^{-3} \text{ kg/kg concrete}$$
(4)

In addition, (Othmen et al., 2018) have previously determined that the marginal law of *C* is lognormal, and (Bonnet et al., 2017) have identified the "measurement error" introduced by the measurement protocol, but only for a mortar. This is distributed according to a generalized extremum law (GeV, (Johnson et al., 1995, p. 3)) with the parameters k = 0.16,  $\sigma = 9.3 \times 10^{-5}$ , and  $\mu =$  $-8.4 \times 10^{-5}$ . (Schoefs et al., 2023) recently pointed out that the error model varies with chloride content. Consequently, the error should vary according to each trajectory. For simplicity, it is

- 419 considered constant in this paper and its standard deviation is an average of the potential standard
- 420 deviations (Schoefs et al., 2023).
- 421 The best-fitting RF model for  $\tilde{Z}$  is then the sum of a log-normal RF Y and a GeV random variable
- 422 W such that, according to Equation (1), Z = Y + W.
- 423 However, the joint PDF of such an RF is not explicit, and SCAP-1D cannot simply be applied.
- 424 The small number of points per trajectory also precludes an estimation by WLSE on semivariograms
- 425 (see 4.1.2).
- 426 Two modeling compromises are then possible in order to ensure explicitness:
- 427 <u>Model a</u>: consider a normal marginal distribution of the total chloride content and a normal
   428 distribution of the error, so that

$$\tilde{Z} \sim \underbrace{N(\mu(x), s. R(a))}_{G^1} + t. N(0, I)$$
(5)

- 429 where *s* and *t* are respectively the empirical variance of the signal and the error and R(a) is the 430 autocorrelation matrix.
- Model b: do not consider the measurement error, whose standard deviation (1.55 × 10<sup>-4</sup> kg/kg
  concrete according to the model of (Bonnet et al., 2017) ) would induce a maximum CoV of 8.3%
  (on trajectory 13; Ext side, z = 75 mm). For this model,

$$\tilde{Z} \sim \underbrace{\exp\left(\underbrace{N(\mu(x), s. R(a))}_{G^2}\right)}^{LN}$$
(6)

434  $G^1$  and LN are then the RF describing the SV of C, with  $G^2$  being the source GRF of LN.

- 435 Since the first approximation of the maximum CoV according to the model of (Bonnet et al., 2017)
- 436 is significant, we then apply SCAP-1D considering both models, so that :
- 437 if the Signal-to-Noise Ratio (SNR) associated with model *a* is low, the error is not negligible and
  438 model *a* is retained;
- 439 if the SNR associated with model *a* is high, the error is considered negligible and model *b* is
  440 retained.

Due to the absence of non-stationarities visible in Figure 12, the trajectories are *a priori* assumed to be stationary, so that  $\mu(x) = \mu$ . We also recall that the model of the autocorrelation function chosen for  $G^1$  and LN is exponential. According to (Chilès & Delfiner, 2012, p. 106), to guarantee such a model for the autocorrelation function  $\rho_{LN}(a)$  of LN, the autocorrelation function  $\rho_{G^2}$  of  $G^2$  must be written as follows

$$\rho_{G^2}(\cdot) = \ln(\rho_{LN}(\cdot), [e-1] + 1) \tag{7}$$

446 According to (Chilès & Delfiner, 2012, p. 106), the mean  $m_{LN}$  and the variance  $\sigma_{LN}^2$  of LN are 447 then written as follows

$$m_{LN} = \exp\left(\mu + \frac{1}{2}s\right)$$
 (8);  $\sigma_{LN}^2 = m_{LN}^2(\exp(s) - 1)$  (9)

# 448 6.3. Model a: Parameter estimation, hypothesis testing and analysis (Steps II, III 449 and IV)

450 Preliminary note: Prior to estimation, the trajectory data, of order  $10^{-3}$ , are multiplied by a factor 451  $k = 10^4$  to facilitate the convergence of the MLE. The estimates of t, s and  $\mu$  are then transformed 452 accordingly before analysis.

453 Table 2 in the Appendix details the results of the SCAP-1D implementation considering model *a*. 454 Only the results of the  $\Sigma_{mod}$ -tests are considered due to the small number of points per trajectory.

Analyzing these, the estimates for trajectories 4 and 5 are rejected due to the rejection of the stationarity hypothesis by the KPSS-test and because the estimated error and practical range are zero and well below  $2\Delta_x$ , respectively, so that the spatial correlation of the data cannot be described (see 4.1.5). This is also the case for trajectories 2, 11, 12, and 13 and can be explained by their very small number of points. Similarly, the estimates associated with trajectories 15, 17, and 24 are not considered because the estimated practical range is zero. Finally, no valid and convergent estimate can be obtained for trajectory 14. As a first step, for each selected trajectory, we plot the estimates 462  $\hat{\mu}$  of the mean with their 95% MLE-based confidence intervals in Figure 13 (a)<sup>5</sup>, and the estimates 463  $\sqrt{\hat{s} + \hat{t}}$  of the standard deviation, for which the distribution is unknown, in Figure 13 (b). We then 464 note that these are consistent with the statistical mean and standard deviation profiles previously 465 plotted in Figure 8: the values estimated by kriging (see 4.1) do not produce perturbations of these 466 statistical estimates.

As a second step, we plot in Figure 14 (a) the standard deviation estimates  $\hat{\tau} = \sqrt{\hat{t}}$  and  $\hat{\sigma} = \sqrt{\hat{s}}$  of 467 the error and signal, respectively, and the standard deviation  $\tau_{\varepsilon}$  associated with the error model of 468 469 (Bonnet et al., 2017). Trajectories for which the estimated error is zero are not considered. We then compare the roots of the experimental SNRs  $\hat{t}/\hat{s}$  and the fixed error SNRs  $\tau_{\varepsilon}^2/\hat{s}$  in Figure 14 (b). 470 Thus, we find that 75% of the roots of the experimental SNRs are greater than 100 and show no 471 472 specific trend along. The fixed error SNRs are logically lower than the experimental ones 473 because the error was calculated for mortars, for which the evaluation error is larger (Schoefs et 474 al., 2023).

Therefore, in accordance with the previously stated modeling assumptions, we consider the measurement error to be negligible and retain model b for the SCAP-1D trajectory study.

#### 477 6.4. Model b: Parameter estimation, hypothesis testing and analysis (Steps II, III, IV)

478 Preliminary note: Since the RF considered in model b is log-normal, the data considered for the 479 SCAP-1D implementation are the logarithms of the debiased data to consider a normal joint PDF. 480 The estimators of  $m_{LN}$ ,  $\sigma_{LN}^2$  and  $r_{95,LN}$  are then calculated from Equations (7) to (9) and from the 481 estimators  $\hat{s}$ ,  $\hat{t}$  and  $\hat{a}$ .

- 482 Table 3 in the Appendix shows the result of the SCAP-1D implementation considering model *b*. Only
- 483 the results of the  $\Sigma_{mod}$ -tests are considered due to the small number of points per trajectory. The
- 484 analysis shows that the estimates for trajectories 3, 4, 5, and 16 are rejected due to the rejection of the

<sup>&</sup>lt;sup>5</sup> some values are missing due to the singularity of the Fisher matrix

485 stationarity hypothesis by the KPSS-test. A model of the mean and covariance function that satisfies 486 the hypothesis cannot be found, so no geostatistical parameter estimates are available for these 487 trajectories. 77% of the estimates are then valid on the Ext side (10/13) and 92% on the Int side 488 (11/12).

The estimates  $\hat{m}_{LN}$  and  $\hat{\sigma}_{LN}$  of the means and standard deviations are plotted with their confidence intervals (CIs) for each trajectory in Figure 15. Note that the estimates are consistent with the statistical mean and standard deviation profiles previously plotted in Figure 8. Meanwhile, the estimates  $\hat{r}_{95,LN}$  of the practical ranges are plotted with their CIs in Figure 16.

In the case of the trajectories associated with the Ext side, only 8 out of 10 estimates retained by SCAP-1D (i.e., 47% of all estimates) are finally retained after filtering out the outliers. These estimates all satisfy the interpretability criterion (see 4.1.5) with values between 100 and 300 cm for a minimum measurement step of 30 cm. However, contrary to the assumptions made at the beginning of the study, there is no trend in the evolution of  $\hat{r}_{95,LN}$  as a function of the depth.

In the case of the trajectories associated with the Int side, all the estimates retained by SCAP-1D can be considered after filtering out the outliers (i.e., 92% of all the estimates). In accordance with the hypotheses formulated at the beginning of the study, we observe a practical range of zero near the surface, then a quasi-constant value around 60 cm-100 cm from 20 mm depth, respecting the interpretability criterion. This tendency reflects a transition from a purely random quantity of interest (QI) at the surface to a spatially correlated QI. at depth. We propose the following explanation:

• At the surface, many spatiotemporally random material (microcracks, ...), physicochemical (local runoff, ...), and environmental phenomena (biofouling, ...) contribute to the supply and convection of chlorides, so that, in coherence with the central limit theorem, the chloride concentration at the surface can be considered completely random, at least at the scale of the 30 cm spatial discretization studied;

at depth, the phenomenon of diffusion and fixation of *C* gradually takes precedence over their
 distribution within the coating, so that the fixation and diffusion properties of the concrete become

predominant with respect to the external and surface phenomena. Since the concrete has a certain
spatial homogeneity, it is then consistent that *C* is spatially correlated.

Furthermore, it is noted that the value of  $\hat{r}_{95,LN}$  at the rebar depth is of the same order as the as the conventional spacing of the vibrations during concrete pouring, from 30 to 60 cm (Guiraud, 2018; SETRA, 2005, p. 199). This observation then reinforces the idea that the spatial correlation of *C* is strongly related to the homogeneity properties of the concrete.

#### 517 6.5. Model b: construction of a unique autocorrelation model of chloride content

518 The results obtained from the study of the Int side trajectories are much more convincing than those 519 obtained from the study of the Ext side trajectories. In order to propose a unique autocorrelation model of C for the diffusion region, we then perform an MLE on the set of reduced centered 520 521 trajectories of the Int side (without the first 10 mm), considering a unique scaling parameter a. Due to the non-stationarity of the mean and variance of C as a function of depth, we consider the 522 523 trajectories independent of each other (we therefore neglect the SV along the z-axis). Still considering an exponential covariance model, the estimated practical range is  $\hat{r}_{95} = 71$  cm, i.e.  $\hat{a} = 24$  cm. 524 525 We then state that the Gaussian model of identical practical range has the scale parameter  $\hat{a} = 41$  cm. However, the calculation of the ER from the AICC does not allow us to distinguish between the two 526 527 models from the data, either by considering each trajectory or by estimating the MLE over all 528 trajectories. In fact, their values are between 1 and 1.29. They are reported in Table 4 in the Appendix. 529 Considering only the estimation results from the Int side, which are considered more robust due to 530 the larger amount of data per trajectory, we can partially answer the questions raised in the 531 introduction of this study. The SV of the chloride content seems indeed to depend on the depth, with 532 a large variability within the convection zone, and much less and more stable in the diffusion zone 533 with a practical range of about 70 cm. However, this value is more than two to three times lower than the practical range values of the average diffusion parameters  $C_{sa}$  and  $D_a$  reported in the 534 literature, between 176 and 264 cm (Engelund & Sørensen, 1998; Karimi, 2002; O'Connor & 535

Kenshel, 2013). However, it should be noted that the latter are obtained with less robust methods than
SCAP-1D and with even smaller data sets.

However, the question of the influence of the exposure conditions on the SV of the chloride content remains open due to the inconsistency of the practical range estimates on the Ext side. However, given the interpretation of the results for the Int side, it can be expected to have an influence only near the surface, where the chloride contents are already spatially decorrelated. However, this suggestion needs to be confirmed by studying an even richer database in order to increase the robustness of the estimates.

# 544 7. Spatial variability estimation of chloride diffusion parameters of J-beam and physical 545 analysis

#### 546 7.1. Geostatistical pre-study and choice of procedure parameters (Step I):

The trajectories of  $\log(C_{sa})$  and  $\log(D_a)$  are shown in Figure 17 to Figure 20, as well as their means and the histograms and experimental semivariogams of centered trajectories. These curves are plotted for each trajectory in three cases: (i) stationary mean (no changepoint); (ii) piecewise stationary mean with one changepoint determined via the PELT algorithm (Killick et al., 2012); (iii) piecewise stationary average at two changepoints determined via the PELT algorithm.

The presence of sills on the experimental semi-variograms allows identifying the *a priori* stationary centered trajectories and thus the most appropriate non-stationarity model for each. The shapes of the histograms and semi-variograms (nugget effect, tangent to the origin, ...) allows identifying models of joint density and a priori autocorrelation function that are adequate to the geostatistical description of the data.

557 Since the amount of data per trajectory is limited, especially for the Ext side, Step I could only 558 be executed for the trajectories  $log(C_{sa})$  1 and  $log(D_a)$  1. The observations made as well as the 559 models considered *a priori* are presented in Table 7.

#### 560 7.1.1. Fields models

Although the histogram of the centered trajectory  $\log(C_{sa})$  1 is asymmetric, this may be due to the spatial correlation of the data. The histogram of  $\log(D_a)$  1 is centered. Moreover, we do not notice any nugget effect. This is due to the fact that the measurement noise of the chloride data is negligible (see 6.3). For simplicity and consistency with the results of the literature, a Gaussian Random Field model (GRF) is used to estimate spatial variability. This model is also retained by default for the  $\log(C_{sa})$  2 and  $\log(D_a)$  2 trajectories.

#### 567 7.1.2. Averaging models: apparent non-stationarity

568 The trajectories are *a priori* globally stationary since the experimental semi-variograms without any 569 changepoint in the mean present a sill. A constant mean model is therefore retained.

#### 570 7.1.3. Covariance function models

571 Due to the large between measurement points, the spatial resolution is insufficient to visualize the 572 presence of a tangent at the origin of the experimental semi-variograms. We therefore retain both the 573 Gaussian and exponential covariance models without *a priori* preference.

This choice does not allow accounting for the correlation between  $log(C_{sa})$  and  $log(D_a)$ , well identified by (Othmen et al., 2018) and which should be modeled via a cross-correlated Random Field (XRF) (Chilès & Delfiner, 2012, p. 332). It is however justified as XRF modelling requires additional parameters (cross-correlation coefficient and cross-scale parameter(s)) which leads to overparameterization and unsuccessful assessment attempts due to the limited amount of data<sup>6</sup>.

#### 579 7.2. Parameter estimation, hypothesis testing and analysis (Steps II, III and IV)

- 580 SCAP-1D steps II and III are applied to the trajectories of  $log(D_a)$  considering the models kept in
- 581 7.1. This allows the most appropriate covariance model to be determined from the calculation of

<sup>&</sup>lt;sup>6</sup> MLE on raw data and MLE on data enriched via Metropolis-Hastings sampling did not give satisfactory results

582 Evidence Ratios (ER). These are computed from the AICC. Table 8 shows the results of SCAP-1D
583 execution by trajectory. The models are ranked by increasing value of their ER.

First no estimate is rejected by the hypothesis tests. The estimates computed from the exponential and Gaussian models are then consistent for each of the trajectories except for the trajectory  $log(C_{sa})$ 2 (relative errors of respectively 21%, 63%, 9% and 21% between the practical ranges of the Gaussian model and those of the exponential model). Furthermore, the exponential and Gaussian covariance models cannot be distinguished from the ERs (maximum difference observed for the  $log(C_{sa})$  1 trajectory with a ratio of 1 for the Gaussian model and a ratio of 2.13 for the exponential model). However, the Gaussian model leads to smaller coefficients of variation of the

591 geostatistical parameters (up to a factor of 2.5 for the scale parameter of trajectory  $log(C_{sa})$  1).

Second, the majority of the practical range estimates do not meet the interpretability criterion (see 4.1.5). All of the estimates of  $r_{95}$  made on the  $\log(D_a)$  trajectories as well as the estimate made on the  $\log(C_{sa})$  2 trajectories are indeed of the order of 30 cm, which is the minimum spacing between measurements.

Therefore, only the estimates made on the  $\log(C_{sa})$  1 trajectory are retained. The estimated practical range is indeed 50 to 63 cm depending on the chosen covariance model, exponential or Gaussian (i.e., a ratio  $r_{95}/L$  of 0.05 to 0.07). The latter can then be preferred *a priori* because of its minimal ER even if exponential model is close.

Thus, due to the too large spacing between measurement points compared to the hidden value of their practical range, the question of whether the SV of the scattering parameters is a function of the exposure conditions cannot be answered here.

Furthermore, the interpretable estimates of practical ranges differ significantly from the 100 to 200 cm values estimated by (O'Connor & Kenshel, 2013) on a bridge exposed to chloride ion penetration. Such a difference can be explained by the fact that these authors significantly enriched their initial kriging database (number of points multiplied by 7) before producing their estimates, which may have influenced them significantly.

#### 608 **8. Conclusion**

In this paper, a new database is proposed for analysis of the spatial variability of chloride ingress into concrete. A total of 460 measurements was available, belonging to 2 exposures zones: 12 trajectories of 21 points for each profile on the Internal side (sheltered from wetting-drying cycles) and 13 trajectories of 16 points on the External side. This data base was uses to analyze the spatial variability of (i) chloride content, (ii) parameters of a model of chloride diffusion. The analysis is performed on the basis of a complete framework (SCAP-1D) which raises, rigorously for the first time, questions of stationarity, marginal distribution and auto-correlation.

616 The application to the total chloride content data showed that:

the measurement error on total chloride content is negligible for the estimation of their spatial
variability;

- near the surface, spatio-temporally random physico-chemical and environmental phenomena
   contribute to the diffusion and convection of chlorides so that, in coherence with the central limit
   theorem, total chloride content at the surface can be considered totally random;
- in depth (from about 20 mm), total chlorides diffusion and fixation gradually take precedence 622 623 over their distribution within the coating, so that fixation and diffusion properties of concrete 624 become predominant compared to the external and surface phenomena. Total chloride content is 625 then spatially correlated with an estimated general practical range of 71 cm (scale parameter of 626 24 cm) which is of the same order as the conventional vibration spacing during concrete pouring 627 (30-60 cm (Guiraud, 2018; SETRA, 2005, p. 199)). This supports the fact that total chloride 628 content spatial correlation in depth is strongly related to the homogeneity properties of the 629 concrete resulting from pouring.

630 The application to the identification of diffusion parameters showed that:

631 The application to the chloride diffusion parameters data was presented in section 7. It allowed 632 estimation of geostatistical parameters only for the logarithm of the average surface chlorides 633 concentration. This is due to a limited number of points per trajectory (from 16 to 21) and a too large spacing between measurement points (30 cm). The estimated practical range value is nevertheless
consistent with that of the total chloride content in depth since it is between 50 and 63 cm.

Implementation of SCAP-1D on these two datasets allowed to note that when the number of points 636 637 per trajectory is limited but not small ( $\geq 20$ ), it is more efficient to first consider the results of statistical tests performed on the standardized trajectory from the empirical covariance matrix, and 638 639 then in a second step those performed on the standardized trajectory from the modeled covariance 640 matrix. This allows to first assess the validity of the mean model, then those of the marginal density 641 model and the estimates of the geostatistical parameters of the source Random Field. On the contrary, when the number of points per trajectory is small (< 20), it is preferable to consider only the results 642 643 of the statistical tests performed on the standardized trajectory from the modeled covariance matrix, 644 in order to guarantee its positivity of the latter.

#### 646 **10. Appendix – Detailed tables of estimation process data**

647 Table 1- Definition of trajectories and proportion of kriged data per trajectory – Ext side and Int side

	kriged data -	Ex side		kriged data - Int side							
trajectory	depth (mm)	number j	percentage	trajectory	depth (mm)	number	percentage				
1	2.5	0	0%	16	2.5	0	0%				
2	7.5	0	0%	17	7.5	1	5%				
3	12.5	0	0%	18	12.5	1	5%				
4	17.5	0	0%	19	17.5	1	5%				
5	22.5	0	0%	20	22.5	2	10%				
6	27.5	0	0%	21	27.5	1	5%				
7	32.5	0	0%	22	32.5	2	10%				
8	37.5	2	13%	23	37.5	2	10%				
9	42.5	3	19%	24	42.5	4	19%				
10	47.5	4	25%	25	47.5	3	14%				
11	55	1	6%	26	55	4	19%				
12	65	1	6%	27	65	4	19%				
13	75	3	19%	28	75	9	43%				
14	85	9	56%	29	85	15	71%				
15	95	10	63%	30	95	17	81%				
	Total	33	14%		Total	66	21%				

648 Table 2 - Estimation of geostatistical parameters of total chloride concentration trajectories by SCAP-1D 649 model a

																·										
		tra	ajectory			mode	el			Σex	o-tests			Σmod	i-tests		estin	nates (da	ita multipl	ied by k=	=10e4)	Coefficient of Variation (CoV)				
	#	Side	z (mm)	Npts	model	cov	reg	chg	chi2	KS	KPSS	DF	chi2	KS	KPSS	DF	μ	s	t	а	r95	CoV μ	CoV s	CoV t	CoV a	
	1	Ext	2,5	16	GW	expon	0	0	0	0	0	1	0	0	0	1	18,66	19,63	0,00	0,01	0,03	NaN	NaN	NaN	NaN	
	2	Ext	7,5	16	GW	expon	0	0	0	0	0	1	0	0	0	1	25,86	64,36	0,00	0,00	0,00	2,0E-01	1,2E+00	5,7E-01	1,7E+00	
	3	Ext	12,5	16	GW	expon	0	0	1	0	1	0	0	0	0	1	40,04	49,27	36,47	2,57	6,28	1,2E-01	8,9E-01	5,3E-01	1,3E+00	
	4	Ext	17,5	16	GW	expon	0	0	0	0	0	1	0	0	1	1	41,59	91,14	0,00	0,01	0,01	5,7E-02	7,6E+02	1,8E+02	3,8E+05	
	5	Ext	22,5	16	GW	expon	0	0	0	0	1	0	0	0	1	1	39,44	96,31	0,00	0,01	0,01	NaN	NaN	NaN	NaN	
	6	Ext	27,5	16	GW	expon	0	0	1	0	0	1	0	0	0	1	36,65	53,47	0,00	0,52	1,55	NaN	NaN	NaN	NaN	
	7	Ext	32,5	16	GW	expon	0	0	1	0	0	1	0	0	0	1	34,95	59,57	0,00	0,32	0,96	NaN	NaN	NaN	NaN	
	8	Ext	37,5	16	GW	expon	0	0	0	0	0	1	0	0	0	1	30,95	62,31	0,00	0,37	1,12	NaN	NaN	NaN	NaN	
	9	Ext	42,5	16	GW	expon	0	0	0	0	0	1	0	0	0	1	29,20	18,89	7,15	2,17	5,80	9,1E-02	7,5E-01	7,5E-01	1,1E+00	
	10	Ext	47,5	16	GW	expon	0	0	0	0	0	1	0	0	0	1	27,69	19,49	0,00	0,71	2,12	NaN	NaN	NaN	NaN	
	11	Ext	55	16	GW	expon	0	0	0	0	0	1	0	0	0	1	20,98	16,46	0,00	0,01	0,02	NaN	NaN	NaN	NaN	
	12	Ext	65	16	GW	expon	0	0	0	1	1	0	0	0	0	1	18,89	11,95	0,00	0,01	0,03	4,6E-02	1,5E+02	9,6E+09	5,6E+05	
	13	Ext	75	16	GW	expon	0	0	0	0	0	1	0	0	0	1	18,61	8,47	0,00	0,01	0,03	3,9E-02	1,9E+02	9,0E+09	1,5E+05	
	14	Int	2,5	21	GW	expon	0	0	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	
	15	Int	7,5	21	GW	expon	0	0	0	0	0	1	0	0	0	1	22,94	135,81	0,00	0,00	0,00	NaN	NaN	NaN	NaN	
	16	Int	12,5	21	GW	expon	0	0	1	1	0	1	0	0	0	1	40,27	38,35	111,41	3,19	5,21	1,2E-01	1,3E+00	3,6E-01	1,5E+00	
	17	Int	17,5	21	GW	expon	0	0	0	0	0	1	0	0	0	1	53,50	180,15	0,00	0,00	0,00	NaN	NaN	NaN	NaN	
	18	Int	22,5	21	GW	expon	0	0	0	0	0	1	0	0	0	1	55,01	150,55	0,00	0,34	1,01	6,5E <b>-</b> 02	1,1E+00	1,8E+07	1,0E+00	
	19	Int	27,5	21	GW	expon	0	0	0	0	0	1	0	0	0	1	50,09	154,45	0,00	0,24	0,72	6,5E-02	1,8E+00	3,2E+05	1,1E+00	
	20	Int	32,5	21	GW	expon	0	0	0	0	0	1	0	0	0	1	47,18	168,62	0,00	0,31	0,93	7,8E-02	9,9E-01	8,2E+07	8,8E-01	
	21	Int	37,5	21	GW	expon	0	0	0	0	0	1	0	0	0	1	39,46	126,85	4,31	0,09	0,26	6,4E-02	2,6E+01	7,8E+02	7,8E+00	
	22	Int	42,5	21	GW	expon	0	0	0	0	1	0	0	0	0	1	39,57	103,23	2,06	0,29	0,86	7,2E-02	1,2E+00	5,9E+01	1,1E+00	
	23	Int	47,5	21	GW	expon	0	0	1	0	0	1	0	0	0	1	34,59	72,53	0,00	0,35	1,06	7,3E-02	9,4E-01	2,7E+07	8,5E-01	
	24	Int	55	21	GW	expon	0	0	0	0	0	1	0	0	0	1	26,46	28,31	0,00	0,00	0,00	NaN	NaN	NaN	NaN	
	25	Int	65	21	GW	expon	0	0	0	0	0	1	0	0	0	1	24,73	25,35	0,00	0,25	0,74	5,4E-02	1,9E+00	6,9E+05	1,2E+00	

650

GW : Gaussian + additive white noise; reg : regression degree of mean model; chg : maximum number of changepoints of mean model

## Table 3 - Estimation of geostatistical parameters of the logarithm of total chloride concentration trajectories by SCAP-1D - model b

Γ		traj	ectory			mode				Σex	p-tests			Σmo	d-tests		estimate	s ( <b>l</b> ogarithr	n of data)	Coefficie	nts of Varia	tion (CoV)
Γ	# F	ace	z (mm)	Npts	modèle	cov	reg	chg	chi2	KS	KPSS	DF	chi2	KS	KPSS	DF	μ	s	а	CoV μ	CoV s	CoV a
Γ	1 E	Ext	2,5	16	G	expon	0	0	0	1	0	1	0	0	0	1	6,31	0,05	0,00	NaN	NaN	NaN
	2 8	Ext	7,5	16	G	expon	0	0	0	1	1	1	0	0	0	1	6,04	0,08	0,52	1,6E-02	4,2E-01	7,5E-01
	3 E	Ext	12,5	16	G	expon	0	0	0	0	0	1	0	0	1	1	5,54	0,06	0,26	1,3E-02	3,7E-01	8,4E-01
	4 E	Ext	17,5	16	G	expon	0	0	0	0	0	1	0	0	1	1	5,51	0,05	0,00	1,0E-02	3,5E-01	1,8E+06
	5 E	Ext	22,5	16	G	expon	0	0	0	0	1	1	0	0	1	1	5,57	0,07	0,13	1,2E-02	3,5E-01	1,6E+00
ſ	6 E	Ext	27,5	16	G	expon	0	0	0	0	1	1	0	0	0	1	5,63	0,05	0,47	1,3E-02	4,0E-01	6,4E-01
	7 E	Ext	32,5	16	G	expon	0	0	0	1	1	1	0	0	0	1	5,68	0,05	0,25	1,1E-02	3,7E-01	9,4E-01
	8 E	Ext	37,5	16	G	expon	0	0	0	0	0	1	0	0	0	1	5,81	0,06	0,40	1,4E-02	3,9E-01	6,7E-01
	9 E	Ext	42,5	16	G	expon	0	0	0	1	1	0	0	0	0	1	5,86	0,03	0,93	1,3E-02	4,8E-01	6,5E-01
	10 E	Ext	47,5	16	G	expon	0	0	0	1	1	1	0	0	0	1	5,90	0,03	0,69	1,1E-02	4,6E-01	7,2E-01
	11 E	Ext	55	16	G	expon	0	0	0	0	0	1	0	0	0	1	6,19	0,05	0,02	8,7E-03	3,5E-01	4,8E+02
	12 E	Ext	65	16	G	expon	0	0	0	0	0	1	0	0	0	1	6,29	0,04	0,01	NaN	NaN	NaN
	13 E	Ext	75	16	G	expon	0	0	0	0	0	1	0	0	0	1	6,30	0,03	0,01	6,5E-03	3,5E-01	2,0E+05
Γ	14	Int	2,5	21	G	expon	0	0	0	0	0	1	0	0	0	1	6,18	0,05	0,00	NaN	NaN	NaN
	15	Int	7,5	21	G	expon	0	0	0	0	0	1	0	0	0	_ 1	6,19	0,21	0,00	NaN	NaN	NaN
	16	Int	12,5	21	G	expon	0	0	0	0	0	1	0	0	1	1	5,58	0,14	0,01	1,5E-02	3,1E-01	1,5E+05
ſ	17	Int	17,5	21	G	expon	0	0	0	0	0	1	0	0	0	1	5,26	0,06	0,16	1,1E-02	3,1E-01	7,8E-01
	18	Int	22,5	21	G	expon	0	0	0	1	0	1	0	0	0	1	5,23	0,05	0,34	1,2E-02	3,4E-01	5,3E-01
	19	Int	27,5	21	G	expon	0	0	0	1	0	1	0	0	0	1	5,32	0,05	0,28	1,2E-02	3,3E-01	5,8E-01
	20	Int	32,5	21	G	expon	0	0	0	1	0	1	0	0	0	1	5,39	0,06	0,28	1,3E-02	3,3E-01	6,0E-01
	21	Int	37,5	21	G	expon	0	0	0	1	0	1	0	0	0	1	5,58	0,08	0,13	1,2E-02	3,1E-01	1,0E+00
	22	Int	42,5	21	G	expon	0	0	0	0	0	1	0	0	0	1	5,57	0,07	0,28	1,3E-02	3,3E-01	5,7E-01
	23	Int	47,5	21	G	expon	0	0	0	1	0	1	0	0	0	1	5,70	0,06	0,30	1,2E-02	3,3E-01	5,7E-01
	24	Int	55	21	G	expon	0	0	0	0	0	1	0	0	0	1	5,95	0,04	0,19	7,9E-03	3,1E-01	6,4E-01
	25	Int	65	21	G	expon	0	0	0	0	0	1	0	0	0	1	6.02	0.04	0.24	84E-03	3 2E-01	5 9E-01

653

G : Gaussian; reg : regression degree of mean model; chg : maximum number of changepoints of mean model

654Table 4 - Computation of the evidence ratios associated with the exponential and Gaussian covariance655models for model b using the corrected Akaike criterion

	tr	ajectory		n	nodel		Corrected Al	kaike criteria	Evidence Ratios			
#	Face	z (mm)	Npts	model reg ch		chg	cov gauss	cov expon	cov gauss	cov expon		
17	Int	17.5	21	G	0	0	-3.21E+01	-3.22E+01	1.04	1.00		
18	Int	22.5	21	G	0	0	-3.89E+01	-3.89E+01	1.00	1.05		
19	Int	27.5	21	G	0	0	-3.59E+01	-3.55E+01	1.00	1.21		
20	Int	32.5	21	G	0	0	-3.05E+01	-3.07E+01	1.11	1.00		
21	Int	37.5	21	G	0	0	-2.39E+01	-2.39E+01	1.00	1.01		
22	Int	42.5	21	G	0	0	-2.89E+01	-2.94E+01	1.29	1.00		
23	Int	47.5	21	G	0	0	-3.14E+01	-3.18E+01	1.18	1.00		
24	Int	55	21	G	0	0	-4.21E+01	-4.18E+01	1.00	1.14		
25	Int	65	21	G	0	0	-4.22E+01	-4.18E+01	1.00	1.26		

G : gaussian

656

#### 658 **11. Data Availability Statement**

- 659 Some or all data, models, or code generated or used during the study are available from the
- 660 corresponding author by request:
- chloride content dataset;
- whole MATLAB code generated during the study.

#### 663 12. Acknowledgment

- The authors would like to thank all the partners of iMAREO2 project: Keops Automation (D. Follut,
- 665 D. Olivier), Université de Nantes (M. Roche), Nantes Saint-Nazaire Port (P. Lijour, M. Labegorre).
- The authors would like to thank the Pays de la Loire region for its financial support.

#### 667 13. References

- Angst, U., Elsener, B., Larsen, C. K., & Vennesland, Ø. (2009). Critical chloride content in
  reinforced concrete—A review. *Cement and Concrete Research*, 39(12), 1122–1138.
  https://doi.org/10.1016/j.cemconres.2009.08.006
- Arnaud, M., & Emery, X. (2000). *Estimation et interpolation spatiale: Méthodes déterministes et méthodes géostatistiques*. Hermès. http://agritrop.cirad.fr/487412/
- Baillargeon, S. (2005). *Le Krigeage*. Faculté des sciences et de génie UNIVERSITÉ LAVAL
   QUÉBEC. https://archimede.mat.ulaval.ca/theses/S-Baillargeon\_05.pdf
- Bastidas-Arteaga, E., & Schoefs, F. (2015). Sustainable maintenance and repair of RC coastal
  structures. *Proceedings of the ICE-Maritime Engineering*, *168*(4), 162–173.
- 677 Biondi, L., Perry, M., McAlorum, J., Vlachakis, C., Hamilton, A., & Lo, G. (2021). Alkali-
- Activated Cement Sensors for Sodium Chloride Monitoring. *IEEE Sensors Journal*, 21(19),
  21197–21204. https://doi.org/10.1109/JSEN.2021.3100582
- 680 Bonnet, S., Schoefs, F., & Salta, M. (2017). Sources of uncertainties for total chloride profile
- 681 measurements in concrete: Quantization and impact on probability assessment of corrosion
- 682 initiation. European Journal of Environmental and Civil Engineering, 1–16.
- 683 https://doi.org/10.1080/19648189.2017.1375997
- Bourreau, L., Bouteiller, V., Schoefs, F., Gaillet, L., Thauvin, B., Schneider, J., & Naar, S. (2019).
   Uncertainty assessment of concrete electrical resistivity measurements on a coastal bridge.
- 686 *Structure and Infrastructure Engineering*, 1–11.
- 687 https://doi.org/10.1080/15732479.2018.1557703

- Bourreau, L., Gaillet, L., Bouteiller, V., Schoefs, F., Thauvin, B., Schneider, J., & Naar, S. (2020).
  Spatial identification of exposure zones of concrete structures exposed to a marine
- 690 environment with respect to reinforcement corrosion. *Structure and Infrastructure*
- 691 Engineering, 16(2), 346–354. https://doi.org/10.1080/15732479.2019.1655072
- BS 6349-1:2000. (2000). *Maritime structures. Code of practice for general criteria* [Norme].
   https://shop.bsigroup.com/ProductDetail/?pid=00000000030055558
- Burnham, K. P., & Anderson, D. R. (2010). *Model selection and multimodel inference: A practical information-theoretic approach* (2. ed., [4. printing]). Springer.
- 696 Chilès, J.-P., & Delfiner, P. (2012). *Geostatistics: Modeling spatial uncertainty* (2. ed). Wiley.
- 697 Clerc, R. (2021). Sur l'estimation de la variabilié spatiale des paramètres de corrosion et sa
   698 necessité pour les plans de maintenance des ouvrages maritimes en béton armé [These de
   699 doctorat, Nantes]. https://www.theses.fr/fr/2021NANT4052
- 700 Clerc, R., Oumouni, M., & Schoefs, F. (2019). SCAP-1D: A Spatial Correlation Assessment
- Procedure from unidimensional discrete data. *Reliability Engineering & System Safety*, 191,
  106498. https://doi.org/10.1016/j.ress.2019.106498
- 703 Cressie, N. A. C. (1993). Statistics for spatial data (Rev. ed). Wiley.
- Der Kiureghian, A., & Ke, J.-B. (1987). The stochastic finite element method in structural
  reliability. In Y. K. Lin, G. I. Schuëller, & P. Spanos (Eds.), *Stochastic Structural Mechanics* (pp. 84–109). Springer Berlin Heidelberg.
- Dickey, D. A., & Fuller, W. A. (1981). Likelihood Ratio Statistics for Autoregressive Time Series
  with a Unit Root. *Econometrica*, 49(4), 1057. https://doi.org/10.2307/1912517
- Engelund, S., & Sørensen, J. D. (1998). A probabilistic model for chloride-ingress and initiation of
  corrosion in reinforced concrete structures. *Structural Safety*, 20(1), 69–89.
- Fares, M., Villain, G., Bonnet, S., Palma Lopes, S., Thauvin, B., & Thiery, M. (2018). Determining
  chloride content profiles in concrete using an electrical resistivity tomography device.
- 713 *Cement and Concrete Composites*, 94, 315–326.
- 714 https://doi.org/10.1016/j.cemconcomp.2018.08.001
- Glasser, F. P., Marchand, J., & Samson, E. (2008). Durability of concrete—Degradation phenomena
  involving detrimental chemical reactions. *Cement and Concrete Research*, *38*(2), 226–246.
  https://doi.org/10.1016/j.cemconres.2007.09.015
- 718 Gomez-Cardenas, C., Sbartaï, Z. M., Balayssac, J. P., Garnier, V., & Breysse, D. (2015). New
- 719 optimization algorithm for optimal spatial sampling during non-destructive testing of
- 720 concrete structures. *Engineering Structures*, 88, 92–99.
- 721 https://doi.org/10.1016/j.engstruct.2015.01.014

722 Guiraud, P. (2018, April). Vibration des bétons. Infociments. 723 https://www.infociments.fr/betons/vibration-des-betons 724 Hill, R. (1963). Elastic properties of reinforced solids: Some theoretical principles. Journal of the 725 Mechanics and Physics of Solids, 11(5), 357-372. https://doi.org/10.1016/0022-726 5096(63)90036-X 727 Hunkeler, D. F., Ungricht, H., & Deillon, F. (2000). Determination of the chloride content of 728 concrete and execution of a Round Robin test in two steps (VSS 546). Eidg. Departement für 729 Umwelt, Verkehr, Energie und Kommunikation (UVEK), Bundesamt für Strassen. 730 https://www.tfb.ch/Htdocs/Files/v/8671.pdf/Publikationsliste/ChloridbestimmungRingversu 731 chBerichtVSSNr.5462000.pdf 732 Johnson, N. L., Balakrishnan, N., & Kotz, S. (1995). Continuous univariate distributions. Vol.2 733 (2ed.). Wiley. 734 Karimi, A. R. (2002). Probabilistic assessment of deterioration and strength of concrete bridge 735 beams and slabs [Imperial College London]. 736 https://ethos.bl.uk/OrderDetails.do?uin=uk.bl.ethos.540519 737 Kaushik, S. K., & Islam, S. (1995). Suitability of sea water for mixing structural concrete exposed 738 to a marine environment. Cement and Concrete Composites, 17(3), 177-185. 739 https://doi.org/10.1016/0958-9465(95)00015-5 740 Killick, R., Fearnhead, P., & Eckley, I. A. (2012). Optimal Detection of Changepoints With a 741 Linear Computational Cost. Journal of the American Statistical Association, 107(500), 742 1590–1598. https://doi.org/10.1080/01621459.2012.737745 743 Kwiatkowski, D., Phillips, P. C. B., Schmidt, P., & Shin, Y. (1992). Testing the null hypothesis of 744 stationarity against the alternative of a unit root: How sure are we that economic time series 745 have a unit root? Journal of Econometrics, 54(1), 159-178. https://doi.org/10.1016/0304-746 4076(92)90104-Y 747 Lecieux, Y., Rozière, E., Gaillard, V., Lupi, C., Leduc, D., Priou, J., Guyard, R., Chevreuil, M., & 748 Schoefs, F. (2019). Monitoring of a Reinforced Concrete Wharf Using Structural Health 749 Monitoring System and Material Testing. Journal of Marine Science and Engineering, 7(4), 750 84. https://doi.org/10.3390/jmse7040084 751 Lecieux, Y., Schoefs, F., Bonnet, S., Lecieux, T., & Lopes, S. P. (2015). Quantification and 752 uncertainty analysis of a structural monitoring device: Detection of chloride in concrete 753 using DC electrical resistivity measurement. Nondestructive Testing and Evaluation, 30(3), 754 216-232. https://doi.org/10.1080/10589759.2015.1029476

- O'Connor, A. J., & Kenshel, O. (2013). Experimental Evaluation of the Scale of Fluctuation for
   Spatial Variability Modeling of Chloride-Induced Reinforced Concrete Corrosion. *Journal of Bridge Engineering*, 18(1), 3–14. https://doi.org/10.1061/(ASCE)BE.1943-5592.0000370
- Othmen, I., Bonnet, S., & Schoefs, F. (2018). Statistical investigation of different analysis methods
   for chloride profiles within a real structure in a marine environment. *Ocean Engineering*,
- 760 *157*, 96–107. https://doi.org/10.1016/j.oceaneng.2018.03.040
- Oumouni, M., & Schoefs, F. (2021a). A Perturbed Markovian process with state-dependent
   increments and measurement uncertainty in degradation modeling. *Computer-Aided Civil and Infrastructure Engineering*, *36*(8), 978–995. https://doi.org/10.1111/mice.12644
- Oumouni, M., & Schoefs, F. (2021b). Spatial variability assessment of structures from adaptive
   NDT measurements. *Structural Safety*, 89, 102052.
- 766 https://doi.org/10.1016/j.strusafe.2020.102052
- Priou, J., Lecieux, Y., Chevreuil, M., Gaillard, V., Lupi, C., Leduc, D., Rozière, E., Guyard, R., &
  Schoefs, F. (2019). In situ DC electrical resistivity mapping performed in a reinforced
  concrete wharf using embedded sensors. *Construction and Building Materials*, *211*, 244–
  260. https://doi.org/10.1016/j.conbuildmat.2019.03.152
- Qu, F., Li, W., Dong, W., Tam, V. W. Y., & Yu, T. (2021). Durability deterioration of concrete
  under marine environment from material to structure: A critical review. *Journal of Building Engineering*, *35*, 102074. https://doi.org/10.1016/j.jobe.2020.102074
- Ragab, A. M., Elgammal, M. A., Hodhod, O. A., & Ahmed, T. E. (2016). Evaluation of field
  concrete deterioration under real conditions of seawater attack. *Construction and Building Materials*, *119*, 130–144. https://doi.org/10.1016/j.conbuildmat.2016.05.014
- Ravahatra, N. R., Duprat, F., Schoefs, F., de Larrard, T., & Bastidas-Arteaga, E. (2017). Assessing
  the Capability of Analytical Carbonation Models to Propagate Uncertainties and Spatial
  Variability of Reinforced Concrete Structures. *Frontiers in Built Environment*, *3*.
- 780 https://doi.org/10.3389/fbuil.2017.00001
- Safhi, A. el M., Benzerzour, M., Rivard, P., Abriak, N.-E., & Ennahal, I. (2019). Development of
   self-compacting mortars based on treated marine sediments. *Journal of Building Engineering*, 22, 252–261. https://doi.org/10.1016/j.jobe.2018.12.024
- Schoefs, F., Awa Zahui Raissa, K., Bonnet, S., & O'Conor, A. J. (2023). Uncertainty quantification
  of semi-destructive testing for chloride content assessment for a concrete bridge in maritime
  environment. *Frontiers in Built Environment*, 9.
- 766 Chvirolinicht. 176/mers in Duit Environment, 9.
- 787 https://www.frontiersin.org/articles/10.3389/fbuil.2023.1130066

- Schoefs, F., Bastidas-Arteaga, E., Tran, T. V., Villain, G., & Derobert, X. (2016). Characterization
  of random fields from NDT measurements: A two stages procedure. *Engineering Structures*, *111*, 312–322. https://doi.org/10.1016/j.engstruct.2015.11.041
- Schoefs, F., Bastidas-Arteaga, E., & Tran, T.-V. (2017). Optimal embedded sensor placement for
  spatial variability assessment of stationary random fields. *Engineering Structures*, *152*, 35–
  44. https://doi.org/10.1016/j.engstruct.2017.08.070
- Schoefs, F., Oumouni, M., Clerc, R., Othmen, I., & Bonnet, S. (2017). Statistical analysis and
  probabilistic modeling of chloride ingress spatial variability in concrete coastal
  infrastructures. *Edition 4, Split, Croatie*, 229–234. https://doi.org/10.5150/cmcm.2017.042
- 797 Schoefs, F., Oumouni, M., Follut, D., Lecieux, Y., Gaillard, V., Lupi, C., & Leduc, D. (2022).
- Added value of monitoring for the maintenance of a reinforced concrete wharf with spatial
  variability of chloride content: A practical implementation. *Structure and Infrastructure*

800 *Engineering*, *0*(0), 1–13. https://doi.org/10.1080/15732479.2022.2077767

- 801 SETRA. (2005). Mémoar—Mémento pour la mise en oeuvre sur ouvrages d'art. Cerema (ex-Setra).
   802 https://www.cerema.fr/fr/centre-ressources/boutique/memoar-memento-mise-oeuvre 803 ouvrages-art
- Stewart, M. G. (2004). Spatial variability of pitting corrosion and its influence on structural fragility
  and reliability of RC beams in flexure. *Structural Safety*, 26(4), 453–470.

806 https://doi.org/10.1016/j.strusafe.2004.03.002

- Stewart, M. G., & Mullard, J. A. (2007). Spatial time-dependent reliability analysis of corrosion
  damage and the timing of first repair for RC structures. *Engineering Structures*, 29(7),
- 809 1457–1464. https://doi.org/10.1016/j.engstruct.2006.09.004
- 810 Stewart, M. G., & Val, D. V. (2003). Multiple Limit States and Expected Failure Costs for
- Beteriorating Reinforced Concrete Bridges. *Journal of Bridge Engineering*, 8(6), 405–415.
  https://doi.org/10.1061/(ASCE)1084-0702(2003)8:6(405)

813 Torres-Luque, M., Bastidas-Arteaga, E., Schoefs, F., Sánchez-Silva, M., & Osma, J. F. (2014).

- 814 Non-destructive methods for measuring chloride ingress into concrete: State-of-the-art and 815 future challenges. *Construction and Building Materials*, 68, 68–81.
- 816 Torres-Luque, M., Osma, J. F., Sánchez-Silva, M., Bastidas-Arteaga, E., & Schoefs, F. (2017).
- 817 Chlordetect: Commercial Calcium Aluminate Based Conductimetric Sensor for Chloride
  818 Presence Detection. *Sensors*, *17*(9), 2099. https://doi.org/10.3390/s17092099
- 819 Vennesland, P. Ø., Climent, M. A., & Andrade, C. (2013). Recommendations of RILEM TC 178-
- 820 TMC: Testing and modelling chloride penetration in concrete. *Materials and Structures*, 8.
- 821 https://doi.org/10.1617/s11527-012-9968-1

- Wasserman, L. (2004). *All of Statistics*. Springer New York. https://doi.org/10.1007/978-0-387 21736-9
- 824 Watanabe, A., Tokuda, S., Mizuta, Y., Miyamoto, S., Nakanishi, T., Furukawa, H., & Minagawa,
- H. (2021). Toward automated non-destructive diagnosis of chloride attack on concrete
  structures by near infrared spectroscopy. *Construction and Building Materials*, *305*, 124796.
- 827 https://doi.org/10.1016/j.conbuildmat.2021.124796
- Wegian, F. M. (2010). Effect of seawater for mixing and curing on structural concrete. *The IES Journal Part A: Civil & Structural Engineering*, *3*(4), 235–243.
- 830 https://doi.org/10.1080/19373260.2010.521048
- Zaki, A., Chai, H. K., Aggelis, D. G., & Alver, N. (2015). Non-Destructive Evaluation for
  Corrosion Monitoring in Concrete: A Review and Capability of Acoustic Emission
- 833 Technique. Sensors, 15(8), Article 8. https://doi.org/10.3390/s150819069

#### 836 14. Embedded Tables

Table 5 - Exponential and Gaussian autocorrelation functions, and relations between their scale parameter,
 fluctuation scale and practical range

Name	Model	θ	$\rho(h=\theta)$	r <sub>95</sub>	$\rho(h = r_{95})$									
exponential	$\rho(h) = \exp\left(-\ h\ /a\right)$	2a	0.135	3a	0.05									
Gaussian	$\rho(h) = \exp\left(-\ h\ ^2/a\right)$	$\sqrt{\pi}a$	0.043	1.73a	0.05									
<i>a</i> : scale parame	<i>a</i> : scale parameter; $\theta$ : fluctuation parameter; $r_{95}$ : practical range;													

 $\rho(\cdot)$ : autocorrelation function

839

840

*Table 6- Numbering of the trajectories of average scattering parameters (J-beam)* 

trajectory	Side	number of measurement points
$\log(C_{sa})$ 1	Int	21
$\log(D_a)$ 1	Int	21
$\log(C_{sa})$ 1	Ext	16
$\log(D_a)$ 2	Ext	16

841

## Table 7- Geostatistical pre-study of the trajectories of the logarithms of the average diffusion parameters – J-beam

		analysis o	of experimental	semi-variograms and hist	ograms	mo	models considered							
trajectory	chgpt(*)	nugget	oscillations	original tangent	histogram shape	average	field	covariance						
$\log(C_{sa})_1$	0	no	no	insufficient resolution	asymmetrical	cste	GRF	Gauss or expon						
$\log(D_a)_1$	0	no	no	insufficient resolution	symmetrical	cste	GRF	Gauss or expon						
$\log(C_{sa})_2$	-	-	-	-	-	cste	GRF	Gauss or expon						
$\log(D_a)_2$	-	-	-	-	-	cste	GRF	Gauss or expon						
(*) : changepo	(*) : changepoints necessary for the experimental semi-variogram to have a sill													

844

Table 8- Geostatistical parameter estimates of the trajectories of the logarithms of the mean scattering
 parameters by SCAP-1D

			model				$\Sigma_{exp}$	,-tests			$\Sigma_{mod}$ -tests			estimates			CoV			model selection		range	
traj	#	model	cov	chg	reg	χ <sup>2</sup>	KS	KPSS	DF	$\chi^2$	кѕ	KPSS	DF	α1	s	<i>a1</i> (cm)	α1	5	a1	AICc	ER	<i>r</i> 95 (cm)	r95/L
$log(C_{1})$ 1	1	GRF	expon	0	0	0	0	0	1	0	0	0	1	-4,82	0,13	17	0,02	0,31	0,81	-14,08	2,13	50	0,05
$\log(\mathcal{L}_{sa})$ 1	2	GRF	gauss	0	0	0	0	0	1	0	0	0	1	-4,81	0,15	37	0,02	0,39	0,33	-15,59	1,00	63	0,07
1(())2	1	GRF	expon	0	0	0	0	0	1	0	0	0	1	-5,20	0,07	19	0,01	0,36	1,20	-19,22	1,00	57	0,06
$\log(c_{sa})$ 2	2	GRF	gauss	0	0	0	0	0	1	0	0	0	1	-5,20	0,07	20	0,01	0,36	0,80	-19,11	1,06	35	0,04
$\log(D_{1})$	1	GRF	expon	0	0	0	0	0	1	0	0	0	1	-27,48	0,31	10	0,00	0,31	1,65	3,78	1,01	29	0,03
$\log(D_a)$ 1	2	GRF	gauss	0	0	0	0	0	1	0	0	0	1	-27,48	0,31	18	0,00	0,31	0,77	3,76	1,00	32	0,03
$\log(D_a)$ 2	1	GRF	expon	0	0	0	0	0	1	0	0	0	1	-27,56	0,29	13	0,01	0,35	1,40	4,19	1,00	40	0,04
	2	GRF	gauss	0	0	0	0	0	1	0	0	0	1	-27,56	0,29	19	0,01	0,35	0,72	4,21	1,01	33	0,04

#### 849 **15. List of Figure Captions**

- 850 Figure 1 Illustration of the geostatistical formalism: GRF *Y*
- 851 Figure 2 Illustration of the geostatistical formalism: noisy GRF Z
- 852 Figure 3 General flowchart of SCAP-1D
- Figure 4 Interpretability of practical range (Gaussian autocorrelograms  $\rho(h)$  with three practical
- 854 range values, measurement step  $\Delta x = 5$ )
- Figure 5 Size and position of rebars of the "J-beam" (distance unit: cm; diameter unit: mm)
- Figure 6 Different views: a) Aerial view, b) Sketch of bridge 3, 4 and 5, c) Beam J, d) Exposed
- 857 (Ext) and sheltered (Int) side of beam J.
- 858 Figure 7 Samples taken for chloride titration (J-beam): a) Locations, b) Cross-section, c) Slices
- 859 Figure 8- Profiles of the evolution of the statistical mean and standard deviation of total chloride
- 860 concentrations according to the depth of embedding
- Figure 9 Total chloride concentration database and illustration of the principle of implementing
- 862 SCAP-1D on total chloride concentration data
- Figure 10 Trajectories and statistical averages of the logarithms of the mean diffusion parameters
- at 28 years (J-beam)
- 865 Figure 11 Raw trajectories and associated statistical means trajectory number / depth
- 866 correspondences presented in Table 5
- 867 Figure 12 Selected trajectories and associated statistical means
- 868 Figure 13 Profiles of the evolution of the mean and standard deviation of total chloride content
- according to the depth model a
- 870 Figure 14 Study of the Signal to Noise Ratio model a
- 871 Figure 15 Profiles of the evolution of the mean and standard deviation of total chloride
- 872 concentrations according to the depth of embedding model b
- 873 Figure 16 Practical range profiles of total chloride concentration trajectories as a function of
- 874 embedment depth model b

- Figure 17- Data and averages of trajectory 1 of log mean surface concentration; histograms of
- 876 trajectories so centered; experimental semivariogams of trajectories so centered
- Figure 18- Data and averages of trajectory 1 of logarithm of mean diffusivity; histograms of
- 878 trajectories so centered; experimental semivariogams of trajectories so centered
- 879 Figure 19- Data and averages of logarithm of mean surface concentration trajectory 2; histograms of
- 880 trajectories so centered; experimental semivariogams of trajectories so centered
- Figure 20- Data and averages of trajectory 2 of logarithm of mean diffusivity; histograms of
- trajectories so centered; experimental semivariogams of trajectories so centered



Figure 1 - Illustration of the geostatistical formalism: GRF Y





Figure 2 - Illustration of the geostatistical formalism: noisy GRF Z



Figure 3 - General flowchart of SCAP-1D



894 Figure 4 - Interpretability of practical range (Gaussian autocorrelograms  $\rho(h)$  with three practical range 895 values, measurement step  $\Delta x = 5$ )

891



896 Figure 5 - Size and position of rebars of the "J-beam" (distance unit: cm; diameter unit: mm)



898

Figure 6 - Different views: a) Aerial view, b) Sketch of bridge 3, 4 and 5, c) Beam J, d) Exposed (Ext) and
sheltered (Int) side of beam J.



903 Figure 7 - Samples taken for chloride titration (J-beam): a) Locations, b) Cross-section, c) Slices



Figure 8- Profiles of the evolution of the statistical mean and standard deviation of total chloride
 concentrations according to the depth of embedding





910 Figure 9 - Total chloride concentration database and illustration of the principle of implementing SCAP-1D

911 *on total chloride concentration data* 



(a) Trajectories and statistical means of logarithms of the 28-year average surface concentrations of total chlorides - beam J



(b) trajectories and statistical means of the logarithms of the mean diffusivities of total chlorides at 28 years - beam J

Figure 10 - Trajectories and statistical averages of the logarithms of the mean diffusion parameters at 28
years (J-beam)



Figure 11 - Raw trajectories and associated statistical means - trajectory number / depth correspondences
 presented in Table 1





Figure 12 - Selected trajectories and associated statistical means



Figure 13 - Profiles of the evolution of the mean and standard deviation of total chloride content according
to the depth - model a





Figure 14 - Study of the Signal to Noise Ratio - model a



Figure 15 - Profiles of the evolution of the mean and standard deviation of total chloride concentrations
 according to the depth of embedding - model b



932

Figure 16 - Practical range profiles of total chloride concentration trajectories as a function of embedment
 depth - model b



Figure 17- Data and averages of trajectory 1 of log mean surface concentration; histograms of trajectories so
 centered; experimental semivariogams of trajectories so centered



936

Figure 18- Data and averages of trajectory 1 of logarithm of mean diffusivity; histograms of trajectories so
 centered; experimental semivariogams of trajectories so centered







Figure 20- Data and averages of trajectory 2 of logarithm of mean diffusivity; histograms of trajectories so
 centered; experimental semivariogams of trajectories so centered