

# Modeling of chloride spatial variability in a reinforced concrete wharf from on-site measurements

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1	Modeling of chloride spatial variability in a reinforced concrete wharf from on-
2	site measurements
3 4 5	Romain CLERC <sup>1</sup> , Franck SCHOEFS <sup>1</sup> , Mestapha OUMOUNI <sup>1</sup> , Inès OTHMEN <sup>2</sup> , Stéphanie BONNET <sup>2</sup>
6	<sup>1</sup> Nantes Université, École Centrale Nantes, CNRS, GeM, UMR 6183, F-44000 Nantes, France
7 8	<sup>2</sup> Nantes Université, École Centrale Nantes, CNRS, GeM, UMR 6183, F-44600 Saint-Nazaire, France
9	* Corresponding author email: <a href="mailto:franck.schoefs@univ-nantes.fr">franck.schoefs@univ-nantes.fr</a>
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14 Abstract

Chloride ingress via diffusion is the main deterioration process of reinforced concrete (RC) structures exposed to marine environment. These structures offer to the environment important lengths or surfaces. Due to the material variability (different concrete batches, vibrations) and exposure variability, the material experiences a spatial variability of the deterioration process. This paper presents the geostatistical analysis of in-situ chloride profiles, leading to the assessment of spatial variability (SV) of both the chloride ingress itself and the parameters of the widely used Fick diffusion law (the average surface chloride content,  $C_{sa}$ , and the average chloride diffusion coefficient  $D_a$ ). 37 chloride profiles measured on both sides of the same spandrel beam of a RC wharf are studied, as well as the related estimates of  $C_{sa}$  and  $D_a$ . From an initial selection of random fields models, the geostatistical analysis consists in assessing model parameters using a procedure which tests both data and model assumptions on the fly (ergodicity, stationarity, random field modelling). Combined to the computation of information criteria for each model, this procedure allows providing relevant geostatistical models for chloride ingress,  $C_{sa}$  and  $D_a$ , which render SV as well as measurement error. It is noticeably found that the error of assessment can be neglected when focusing on SV for the range of chloride content studied in the paper. The SV of chloride content seems to depend on the depth, with a large variability within the convection zone, and much less and more stable in the diffusion zone with a practical range of about 70cm. This order of magnitude is consistent with the range of SV computed for  $C_{sa}$  (50 tà 73 cm).

- 31 **Keywords**: Chloride ingress, Reinforced concrete, Marine environment, Spatial variability,
- 32 Random field, Statistical analysis.

#### 1. Introduction

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Chloride ingress is among the main causes of degradation of reinforced concrete (RC) structures where the deterioration is more perceptible in marine environment. That leads to major maintenance costs (Bastidas-Arteaga & Schoefs, 2015; Stewart & Val, 2003). These structures offer to the environment important lengths or surfaces. Due to the variabilities of both the material (different concrete batches, vibrations) and exposure, they experience a spatial variability of their deterioration process. Accounting for realistic spatial correlation of the different parameters in structural safety models may lead to more realistic estimates on structural reliability and maintenance planning, knowing that reliability assessment is affected by spatial correlation (Schoefs et al., 2022; Stewart, 2004; Stewart & Mullard, 2007). Moreover, quantifying this property allows optimizing inspection meshing on a structure (Gomez-Cardenas et al., 2015; Oumouni & Schoefs, 2021b; Schoefs et al.,

44 2017a).

- The variability of the degradation parameters and their measurements can be described at different scales:
- the point scale, characterizing the repeatability of measurements;
  - the local scale of the material, characterizing the heterogeneity of the material for a scale lower than the Representative Elementary Volumes (REV) associated with each of the quantities of interest (Hill, 1963);
    - higher scales (structural elements, structure), characterizing the degree of spatial
      heterogeneity of the material and/or environmental conditions (exposure, loading, ...) for
      scales above the REVs.

The restitution of point and local variabilities requires a statistical modeling, translating the hypothesis of independence, respectively on (i) the measurements of parameters between distinct measurements and for a same measurement point and (ii) the parameter values between similar structures for the same REV dimension. The spatial variability (taking into account local and larger scales) cannot be solely captured by a statistical modelling because of the hypothesis of the existence of a certain degree of heterogeneity, i.e. spatial correlation, between measurements or between parameter values associated with different measurement points or REV on the same structure. This can only be translated by geostatistical modeling. The major challenge of those models is to determine stochastic properties of the Random Fields (RF), namely their marginal distributions, stationarity and the spatial correlations.

Chloride ingress has been investigated deeply since the 70's and it has been shown how complex is the process physically and chemically (Glasser et al., 2008; Kaushik & Islam, 1995; Ragab et al., 2016; Safhi et al., 2019; Wegian, 2010) and the role the material complexity (porosity) is playing (Qu et al., 2021). For the moment, available models of transfer and knowledge don't allow

- to represent accurately the spatial variability of pollutant in concrete (Ravahatra et al., 2017). That is the main reason why direct or indirect assessment of chlorides in situ is the only way to measure, analyze and model the spatial variability of chloride ingress. Then, two kinds of quantities of interest can be considered:
- the total chloride content *C* at several depths;

• their mean diffusion parameters  $C_{sa}$  and  $D_a$ , estimated from profiles of chloride content.

During the last decades sensors have experiences significant progresses (Biondi et al., 2021; Torres-Luque et al., 2014, 2017; Watanabe et al., 2021; Zaki et al., 2015) but there are few concepts that measure the spatial distribution of chloride ingress (Fares et al., 2018; Lecieux et al., 2015, 2019) and their implementation is rare (Priou et al., 2019) even if their interest in terms of the value of information they provide was shown (Schoefs et al., 2022) especially because they allow to filter the signal (Oumouni & Schoefs, 2021a). Moreover, these sensors have to be installed at the building stage and are not suitable for existing structures.

That is the main reason why spatial variability is assessed through Non-Destructive Testing (NDT) tools or Semi-Destructive Techniques (SDT). On-site indirect assessment of chlorides through NDT-techniques is affected by errors of assessment and sensitivity to other parameters (humidity, temperature) which makes it difficult to assess the spatial variability of chlorides (Bonnet et al., 2017; Bourreau et al., 2019; Schoefs et al., 2023). This error affects the optimization of measurements for assessing spatial variability (Oumouni & Schoefs, 2021b). That is why, the rare studies are based on the semi destructive techniques (SDT) i.e. direct weighting of chlorides from cores (O'Connor & Kenshel, 2013; Othmen et al., 2018) reviewed 10 published studies from 2004 to 2016 with only 3 concerning more than 30 measurements and in all cases the distance between measurement was too large in view to assess the spatial correlation. Two main quantities of interest are investigated: chloride content at a given depth and parameters of a model of chloride ingress after post-processing of chloride profiles.

Because of the complexity and cost for making a large amount of cores in a beam, only a few amounts of data is available for each and the distance between cores is too large: 5 cores with a distance of about 1 m in the study of (O'Connor & Kenshel, 2013). That leads to a significant numerical error of assessment of the spatial correlation (Schoefs et al., 2017b). Moreover, the underlying assumptions for RF modelling that are stationarity and ergodicity are not checked. Finally, the error of measurement is usually not considered when identifying correlations. That is a drawback because it is well known that SDT measurements for existing structures (Schoefs et al., 2023) and even for concrete in laboratory (Bonnet et al., 2017; Hunkeler et al., 2000) are affected by a significant error. To overcome these limits, this paper relies on two pillars: first, a unique database and second, a geostatistical analysis that allows to provide geostatistical parameters (namely spatial correlation assessments as well as measurement error) by using a recent method of RF identification (SCAP1D, Clerc et al. 2019), that accounts for uncertainty of measurements, checks ergodicity and accounts for the non-stationarity of the RF.

Concerning the first pillar, marine structures can be exposed in various environments: atmospheric, spatter or splash, tidal and underwater (BS 6349-1:2000, 2000) which were defined according to the tide range (Bourreau et al., 2020). In this work, 30 cores carried out every 30 cm on the same 28-year beam (Othmen et al., 2018) are analyzed. The beam is located in the worst environment that is splash zone (Angst et al., 2009), and the cores are through its width giving access to profiles from each side i.e., exposure conditions (exposed, sheltered).

Concerning the second pillar, two objectives are targeted: the assessment of spatial variability of the total chloride content C, depending on depth and exposure on one hand and the assessment of the spatial variability of the mean diffusion parameters  $C_{sa}$  and  $D_a$  on the second one. First, taking benefit of the availability of C at several depth, the objective is to provide estimates of its geostatistical parameters and to answer the following questions: (i) Is the spatial variability (SV) of C a function of depth? (ii) Is the SV of C a function of the exposure conditions? Second, knowing the age of the structure (28-year), geostatistical parameters of mean diffusion parameters are estimated to answer

the following question: Is the SV of  $C_{sa}$  and  $D_a$  a function of the exposure conditions? The latter question arises because  $C_{sa}$  and  $D_a$  are directly dependent on the chloride profiles, which vary markedly between the exposed and sheltered faces.

Section 2 introduces the geostatistical formalism in a theoretical way before presenting a state of the art of RF modeling, simulation methods and spatial variability estimation methods. An attention is paid to the underlying assumptions of the estimation methods and to the consequences of their neglect on the estimated spatial variability, especially in the frequent case of non-stationary measurements due to spatially variable exposure conditions. Section 3 presents the structure and the available data. Sections 4 and 5 give the results at two levels: total chloride contents, and mean diffusion parameters of total chlorides. Section 6 concludes the paper with some extension to other structures or other exposures.

#### 2. Geostatistical methods for spatial variability assessment of concrete structures

#### 2.1. Modelling of spatial variability

- 132 2.1.1. Random Field modeling
- Practical modelling of spatial variability implies to model spatially variable quantity of interest (q.i)
- as Second Order Intrinsic Stationary Random Fields (RF) (Chilès & Delfiner, 2012). Following
- geostatistical formalism, such a RF Z is defined on a domain D of  $\mathbb{R}^n$  and on a probability space
- 136  $(\Omega, A, P)$  such that:

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- 137  $\forall x \in D^n$  and  $\forall (p,q) \in [1;n]^2$ ,  $Z(x,\cdot) = Z_x$  is a multivariate random variable of  $\omega \in \Omega$  with joint
- probability density function (pdf)  $f_Z(\mathbf{m}, \mathbf{\Sigma})$ ;
- 139  $\forall x \in D^n$  and  $\forall \omega \in \Omega$ ,  $Z(x, \omega) = Z_x^{\omega}$  is a realization of Z georeferenced by vector x, also called
- *trajectory* (in practice, one set of measurements from a unique structure and at a unique time).
- 141  $\forall (h,x) \in D^2$ ,  $\mathbb{E}[Z_{x+h} Z_x] = 0$  and  $Var(Z_{x+h} Z_x) = 2\gamma(h)$  with  $\gamma(\cdot)$  the semi-variogram of
- 142 Z and h any space lag;

143 This definition reflects both the variability of input parameter between structures and its spatial variability within the same structure, which are embedded into the *covariance matrix*  $\Sigma$  and the semi-145 variogram.

146 In this paper, data are modeled as (i) unidimensional (ii) noisy (iii) trend-stationary (iv) Gaussian Random Field (GRF). Indeed, (i) data are unidimensional along beam sides; (ii) if the spacing 147 148 between measurements is larger than the REV, the RF model must be able to reproduce the point and 149 local variabilities, which do not have a correlation structure and can be modeled by white noise; (iii) 150 materials and environmental conditions may vary along beam sides. It is then highly probable that 151 the mean of the quantities of interest varies with space; (iv) GRF are very handy as they allow to 152 model most natural variabilities (following normal or lognormal marginal laws) with only a model 153 for the mean and the covariance (Chilès & Delfiner, 2012).

154 Mathematically,  $\forall x \in D^n$  we thus consider Z such that

$$Z_{x} = \boldsymbol{m}_{x} + W_{x} + Y_{x} \tag{1}$$

With  $m_x$  a deterministic trend,  $W_x \sim N(\mathbf{0}, t \times \mathbf{1}_n)$  a white noise of variance t;  $Y_x \sim N(\mathbf{0}, \Sigma_Y)$  a centered GRF, i.e  $Y_x$  a multivariate normal variable such that

$$\forall (h,x) \in D^2, \Sigma_{Y_{x+h,x}} \equiv Cov(Y_{x+h},Y_x) \equiv C_Y(h) \equiv s \times \rho_Y(h) \qquad (2)$$

- with  $C_Y(\cdot)$  the covariance function of Y, s its variance,  $\rho_Y(\cdot)$  its autocorrelation function, and h any distance-lag between data (equivalences due to the Second Order Stationarity of GRF).
- 159 This formalism is illustrated on Figure 1 and Figure 2, where trajectories, correlation diagrams,
- autocorrelation functions and semi-variograms are plotted for Y and Z respectively, defined such that
- 161 (1)  $Y_x \sim N(\mathbf{0}, \mathbf{\Sigma}_Y)$  with  $\Sigma_{Y_{x+h,x}} = s \times \exp(-1.73 ||h||^2 / r_{95})$ , s = 10 and  $r_{95} = 20$ , and (2) Z = W + 100
- 162 Y with  $W_x \sim N(\mathbf{0}, t \times \mathbf{1}_n)$  and t = 5.

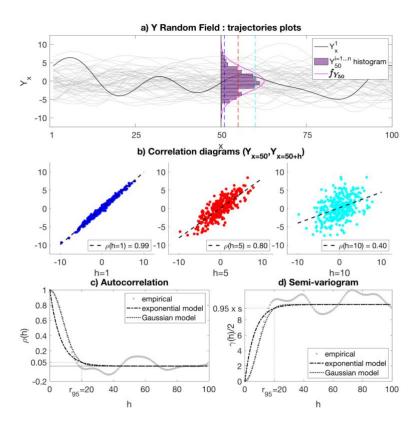


Figure 1 - Illustration of the geostatistical formalism: GRF Y

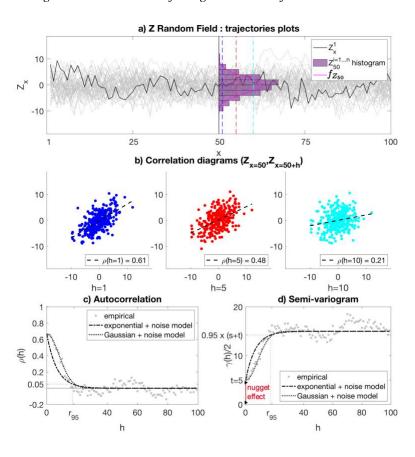


Figure 2 - Illustration of the geostatistical formalism: noisy GRF Z

#### 2.1.2. Autocorrelation models and parameters

Table 1 and Figure 1 respectively presents and illustrates the exponential and Gaussian autocorrelation functions, widely used in the literature. In accordance with Equation (2) such functions are positive definite such that they asymptotically decrease as a function of the distance-lag h. The evolution of this decay is mainly driven by the value of their scale parameter a > 0, which parametrizes the extent of the spatial correlation, i.e. the spacing threshold below which the q.i are strongly correlated.

The distance for which events are weakly correlated is called the Inspection Distance Threshold with a definition of a Spatial Correlation Threshold of 30% in (Schoefs et al., 2016). Several notations and parameters coexist in the literature to describe this threshold, sometimes with some confusion. In the case of isotropic fields, (i) Cressie (1993, p. 118) defines the fluctuation range  $\theta$ , such that  $\theta = \int_{-\infty}^{+\infty} \rho(h)dh$ ; (ii) the same author (p. 88) defines by the practical range  $r_{95}$ , such that  $\rho(r_{95}) = 5\%$  (see Figure 1 and Figure 2) and (iii) the correlation length  $l_c$  is not strictly defined and alternatively taken equal to a,  $\theta$ ,  $r_{95}$  or any other particular range. In order to avoid any confusion and to simplify comparison and interpretations, we specify in Table 1 the expression of  $\theta$  and  $r_{95}$  as a function of a for the exponential and Gaussian models and the associated autocorrelation values  $\rho$ . Note that on

Table 1 - Exponential and Gaussian autocorrelation functions, and relations between their scale parameter, fluctuation scale and practical range

Name	Model	$\theta$	$\rho(h=\theta)$	$r_{95}$	$\rho(h=r_{95})$
exponential	$\rho(h) = \exp\left(-\ h\ /a\right)$	2a	0.135	3 <i>a</i>	0.05
Gaussian	$\rho(h) = \exp\left(-\ h\ ^2/a\right)$	$\sqrt{\pi}a$	0.043	1.73a	0.05

a: scale parameter;  $\theta$ : fluctuation parameter;  $r_{95}$ : practical range;

Figure 1 *Y* is obtained from a Gaussian autocorrelation function.

 $<sup>\</sup>rho(\cdot)$ : autocorrelation function

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2.2.1. Empirical identification

190 RF models behind a trajectory can be identified in case of its *ergodicity*, which can be seen as a 191 mixture of stationarity and asymptotic independence. In that case, special variability can be assessed 192 from empirical representations of the semi-variogram, the covariance function, or the autocorrelation 193 function, respectively called empirical semi-variogram, covariogram and autocorrelogram. 194 Semi-variogram, noted  $\hat{\gamma}(h)$ , is used as its estimate does not require supplementary ones (like mean 195 or variance), and it allows to visually identify (i) Second-Order Stationarity property, (ii) ergodicity property, and (iii) presence and magnitude of local variabilities. Indeed, (i)  $\hat{\gamma}(h)$  asymptotically tends 196 197 toward variance for Second-Order Stationary RF whereas it increases with h otherwise; (ii) ergocidity

is verified if  $\hat{\gamma}(h)$  reaches an asymptote in the limit of half of the domain D (Cressie, 1993, p. 57); (iii) white noise is easily identified by a *nugget effect*, i.e a non-zero value for h = 0 (see Figure 2) Common estimator of  $\hat{\gamma}(h)$  is defined by Equation (3) for a given trajectory  $Z_x^{\omega}$ , where  $S_h$  is the set

of pairs  $(x_p, x_q)$  of  $D^2$  distant from h modulo a given tolerance and  $N_h$  is the cardinal of  $S_h$ .

$$\hat{\gamma}(h) = \frac{1}{2N_h} \sum_{(x_p, x_q) \in S_h} \left( Z_{x_p}^{\omega} - Z_{x_q}^{\omega} \right)^2 \tag{3}$$

We point out that when h becomes close to the dimension of D, statistical inference is no longer valid because the number of pairs of points distant from h is very small. Therefore, geostatisticians recommend plotting this graph only for spacings h such that  $N_h \geq 30$  (Cressie, 1993). In practice, in Civil Engineering, several authors recommend limiting to values of h less than half of the size of the domain (Arnaud & Emery, 2000, Schoefs et al., 2016).

#### 2.2.2. Estimates of model parameters

Once a RF model is chosen from empirical identification, parameters *p* have to be estimated. Several methods are available:

- Maximum Likelihood Estimation (MLE), which allows to estimate *p* from raw data given a RF marginal distribution model, and as the following properties: (i) asymptotically unbiased, (ii) consistent, (iii) equivariant, (iv) asymptotically efficient, and (v) asymptotically normal
- Least-Square Estimate (LSE), which allows to estimate **p** from semi-variogram, covariogram or
- Weighted LSE (wLSE), similar to LSE, each of the residues being weighted by its variance. This allows increasing the weight of the residuals computed from many data (low distance-lag *h*
- 219 Usually, LSE and wLSE fit well the covariograms and the use of wLSE implies a better description 220 of these for small spacing, associated with more data<sup>1</sup>. However, covariograms are not necessarily 221 representative of the effective covariance parameters of the field, which can lead to significant relative 222 errors in their estimation. Because it only considers raw data, MLE provides more accurate estimates, 223 with smaler relative errors. The counterpart is the computation time, since MLE is more time-224 consuming than wLSE and LSE, up to 10 times faster. Thus, and since it is a priori complex to have 225 an initial idea of the scale parameter, a two-phase estimation method of Second-Order Stationary RF 226 parameters is prefered:
- 1. wLSE pre-estimation of the parameters with the following initial values: (i) for s, the empirical variance; (ii) for a, a value such that the fluctuation scale is less than a quarter of the maximum lag-distance (so as to respect the ergodic assumption);
- 230 2. MLE estimation of the parameters with wLSE estimates as initial values.
- This approach allows to obtain accurate estimates while minimizing the cost of the MLE due to the definition of initial values close to the real values of the parameters (Clerc et al., 2019, para. 4.4).

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(Wasserman, 2004, para. 9.4);

values, for which  $N_h$  is greater).

autocorrelogram given models of these;

<sup>&</sup>lt;sup>1</sup> In this case  $\gamma(h) = 1 - C(h)$ , so estimates on covariogram or semi-variogram are equivalent.

#### 233 2.2.3. Choice of best RF model

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Although plotting empirical semi-variogram allows to choose a RF model, its estimator nature induces an imperfect representation of the SV. This is all the truer when the number of measurements is small. Thus, when there is insufficient scientific evidence to support a specific RF model for a q.i, which is the case for chloride content and diffusion parameters (Gomez-Cardenas et al., 2015), we can rely on data-based methods of model choice.

In the following, the Akaike information criterion (AIC) is preferred, as it is much more robust than the  $R^2$  coefficient of determination (Burnham & Anderson, 2010). The AIC allows to compare, with the parsimony criterion, the relevance of any RF model  $M_Z$  with K parameters  $p_{1,\dots,K}$  to describe the hidden RF Z at the origin of a set of n data  $Z_x^\omega$  (for example a trajectory). This criterion assumes that the best model is the one that allows the most accurate description with the fewest possible parameters (Burnham & Anderson, 2010, para. 1.4).

When n/K < 40 and (which is the case hereafter) we use in particular the corrected information criterion (AICc). When  $\boldsymbol{p}$  estimate  $\hat{\boldsymbol{p}}$  is computed with MLE, it writes:

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$$AIC_c = \underbrace{-2\ln(L(\hat{p}|Z_x^{\omega})) + 2K}_{AIC} + \underbrace{\frac{2K(K+1)}{n-K-1}}_{correction}$$

models is then the one that minimizes the AICc, i.e the one that has both a high goodness of fit (high likelihood) and a limited complexity (limited number of parameters).

In practice, the easiest way to compare the relevance of models based on their AICc is via the Evidence Ratio (ER) (Burnham & Anderson, 2010, para. 2.10). This is defined such that, for any model  $M_{Z,i}$  among  $N_m$  models,

With  $L(\widehat{p}|Z_x^{\omega})$  the likelihood of  $\widehat{p}$  with respect to the data. The best model among a set of considered

$$ER_{i} = \exp\left(\frac{1}{2}\left[AICc_{i} - \min_{\substack{j=1,\dots,N_{m} \text{best a priori model}}}(AICc_{j})\right]\right)$$

The ER is therefore 1 for the best *a priori* model and grows exponentially as the models become less and less adapted. From expert knowledge an ER of 2 for a given model does not justify giving preference to the best a priori model over it, whereas an ER of 18 clearly favors the best a priori model (Burnham & Anderson, 2010, para. 2.10).

2.2.4. SCAP1D: a robust estimation procedure

The methods presented are supposes to get an adequate RF model to avoid estimates errors: without modeling precaution, they are only valid under the condition that the data can be described by an SSO and ergodic RF. However, since classical regression techniques coupled with generic autocorrelation models can easily approximate autocorrelograms and covariograms, the estimates of RF model parameters are generally carried out by LSE on raw data without prior verification of these two hypotheses (O'Connor & Kenshel, 2013; Ravahatra et al., 2017; Schoefs et al., 2016). Such a procedure can then lead to important errors, in particular on the estimation of the scale parameter a, and in particular in the cases of non-stationarity in the mean and/or of important measurement noise (Clerc et al., 2019)

To prevent these errors, we use here the SCAP-1D procedure<sup>2</sup> (Clerc et al., 2019). Based on previously presented tools, it allows, from a single trajectory with few measurement points, the estimation of parameters of a unidimensional noisy Trend-Stationary RF with piecewise polynomial mean, constant variance and constant autocorrelation model.

The concept behind SCAP-1D is to ensure the reliability of the parameters' estimates by testing and validating the following hypotheses:

- *H1*: validity of the RF mean model and its parameter estimates; i.e. Second-Order Stationarity of centered RF;
- *H2*: validity of both the joint pdf and autocorrelation function models of the RF and of the estimation of their parameters; i.e., normality of the standard source GRF, obtained by isoprobabilistic transformation of the RF from estimates considering the *a priori* model;

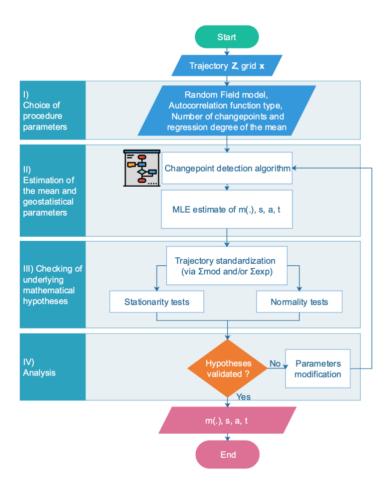
<sup>2</sup> Spatial Corelation Assessment Procedure for Unidimensional Data

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- #3: ergodicity of the trajectory; i.e., non-correlation of the data at infinity or determination of all
   geostatistical parameters from this trajectory.
- The SCAP-1D flowchart is shown in Figure 3. It is organized into four successive steps, which may be repeated when the estimates do not validate hypotheses on the RF model:
- <u>Step I) Choice of SCAP-1D parameters</u>, namely piecewise polynomial mean model (regression degree, number of changepoints), RF joint pdf and autocorrelation function. We recommend to plot the trajectory, its histogram and its experimental semivariogram to facilitate it.
- <u>Step II) Estimation of the mean and geostatistical parameters</u>: changepoints in the mean and their significancy are determined using PELT (Pruned Exact Linear Time) optimal partitioning algorithm of (Killick et al., 2012). Estimation of RF parameters is then performed by the approach presented in 2.2.2<sup>3</sup>.
- Step III) Checking of underlying mathematical hypotheses: plausibility of estimated model is checked by checking ergodicity and performing stationarity tests (DF-test (Dickey & Fuller, 1981), KPSS-test (Kwiatkowski et al., 1992)) and normality tests (χ2, KS-test (Kolmogorov-Smirnov)) on standardized trajectory, based on Step II estimates.
- <u>Step IV) Analysis</u>: based on Step III tests, model chosen in Step I and estimates made in Step II are either accepted (algorithm ends) or rejected (modification on Step I parameters is required).

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<sup>&</sup>lt;sup>3</sup> or by wLSE on the experimental semi-variogram in the case of non-explicit density models.



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Figure 3 - General flowchart of SCAP-1D

- 299 In Step III, standardization of the trajectory is done by Cholesky transformation considering either:
- the empirical covariance matrix: in that case, we only test H1 as tests results only depend on data and estimates of the mean. Tests are then called  $\Sigma_{exp}$ -tests;
- or the modeled covariance matrix: in that case, we test H1 to H3 together as tests results depend on the whole estimates. Tests are then called  $\Sigma_{mod}$ -tests.

#### 2.2.5. Interpretability criterion

In step IV of SCAP-1D, estimates must be analyzed for their physical consistency. In particular, an interpretability criterion can be defined for the estimated practical range. The point is to ensure, with respect to the spacing  $\Delta x$  between measurements, that the variability thus described is indeed a spatial variability and not a local or point variability. This is illustrated in Figure 4, where the Gaussian autocorrelogram is plotted on a measurement grid of step  $\Delta x = 5$  for three practical range values  $(r_{95} < 2\Delta x, r_{95} \simeq \Delta x, \text{ and } r_{95} > 2\Delta x)$ :

• when  $r_{95} \ge 2\Delta x$  (curves c3 and c2): the variability described is spatial variability since there is at least one significant intermediate spatial correlation value between a point's total correlation with itself and the minimal 5% correlation associated with the practical range;

• when  $r_{95} < 2\Delta x$  (curve c1): the variability described may be local or point variability since there is no significant intermediate spatial correlation value to account for spatial variability.

Therefore, care will also be taken to ensure that the criterion  $r_{95} \ge 2\Delta x$  is met when applying SCAP-1D<sup>4</sup>.

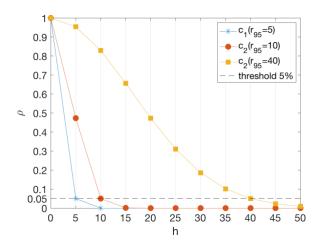


Figure 4 - Interpretability of practical range, measurement step  $\Delta x = 5$ 

#### 3. Presentation of the structure and the measurements

#### 3.1. Presentation of the wharf, the beam and its material

The reinforced concrete beam inspected in the framework of the APOS research operation is a T hyperstatic beam named "J beam" whose dimensions and reinforcement are presented in Figure 5. It is located on lane 51 of the gangway 5 of the coal terminal of Montoir-de-Bretagne (Loire-Atlantique department, France) with GPS coordinates: 47.32°N, -2.17°E. This terminal, built between 1981 and 1983 on the banks of the Loire and less than 7km from the Atlantic Ocean, is 234.4m long and 20.7m

<sup>&</sup>lt;sup>4</sup> This criterion is constructed similarly to that of (Der Kiureghian & Ke, 1987) on the ability of a discretization grid to render the spatial variability properties of a RF.

wide and is located at +8.40m NGF (Figure 6). This proximity to the ocean implies the existence of a tidal phenomenon with an amplitude of 5.80m and a salinity of the water equivalent to maritime conditions at high tide (about 30g Cl<sup>-</sup>/L). Thus, at the highest tidal coefficients, the lower face of beam J, located 5.80m above the Loire level, is in contact with the water about 2 days per month. Finally, one of the particularities of the J-beam is its location on the edge of the quay. It has a side exposed to the wetting-drying cycles, noted Ext, and a sheltered side, noted Int (see Figure 6).

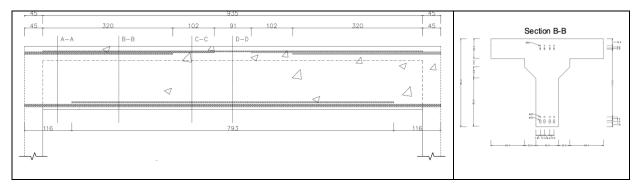


Figure 5 - Size and position of rebars of the "J-beam"

Data from the construction archives indicate that the concrete used to build the structure is a Portland cement concrete CEMI 42.5N dosed at 350kg/m3. The mass fraction of  $m_{cl}$  clinker is thus higher than 90% and that of secondary constituents such as gypsum does not exceed 5%. The aggregates used are sand 0/6, and gravel 5/10 and 10/25. Concrete water porosity  $\varepsilon$ , measured on the central part of 5 specimens used for chloride titration was 13.7% in average and minimum and maximum values were 11.3% and 15.9% respectively. The average compressive strength  $f_{cm}$ , measured on 4 specimens of 5 cm diameter and 10 cm length was 43.5MPa, with minimum and maximum values of 38.5 MPa and 48.9 MPa, respectively.

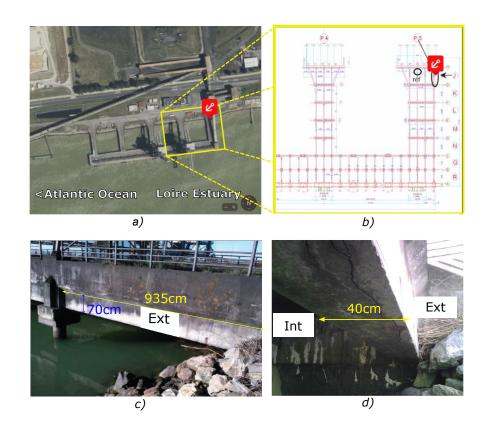


Figure 6 - Different views: a) Aerial view, b) Sketch of bridge 3, 4 and 5, c) Beam J, d) Exposed (Ext) and sheltered (Int) side of beam J.

#### 3.2. Total chloride content measurements at 28 years

#### 3.2.1. Available data set

Chloride content measurements were performed on 30 through cylindrical cores extracted on the J-beam at the same height of 30 cm and with lateral spacing of 30 cm (see Figure 7) (Othmen et al., 2018). Extraction was carried out in 2011-2012, after 28 years of exposure. From initial dimensions of 5 cm in diameter and 40 cm in length, each core was divided into 5 slices (identified on Figure 7). Slices 1, 2, 4 and 5 were used to determine the chloride profiles, using the procedure recommended by the RILEM TC 178-TMC (Vennesland et al., 2013). Slices 1 and 2 are associated to the exterior side (gathered in Ext specimens) and slices 4 and 5 to the interior side (gathered in Int specimens). Whereas deep slices 2 and 4 were 40-mm thick with measures in 10 mm increments, slices 1 and 2 were 45mm-thick with measures in 5-mm increments to catch the value at the surface. Slice 3 was used for further investigation such as porosity, compressive strength and estimation of the initial chloride content. Out of a total of 30 specimens per side, 16 were usable on the Ext Face (53%), and

21 on the Int Face (70%). Moreover, out of the 555 total chloride content from the 30 specimens thus considered, only 460 were usable (83%, i.e., 51% of all the measurements initially planned).

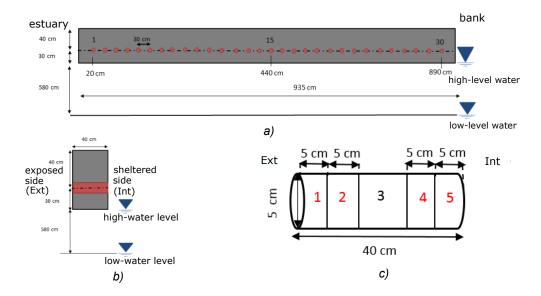


Figure 7 - Samples taken for chloride titration (J-beam): a) Locations, b) Cross-section, c) Slices

Figure 9 hereafter represents then the set of remaining measurements of total chloride content at 28 years finally considered (noted C in the following). Evolutions of empirical mean and standard deviation by depth and by face are represented on Figure 8.

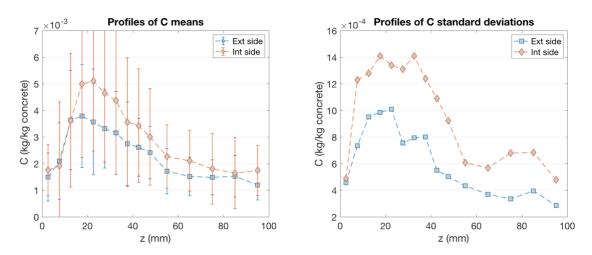


Figure 8- Profiles of the evolution of the statistical mean and standard deviation of total chloride concentrations according to the depth of embedding

#### 3.2.2. Constitution of trajectories

In the following, the evolution of the SV of C is analyzed as a function of depth and exposure conditions. SCAP-1D is then applied to horizontal trajectories of measurements performed on the same face and at the same embedding depth z. The estimation of geostatistical parameters for each trajectory allows then plotting their evolution as a function of z and exposure face. This study principle is illustrated in Figure 9.

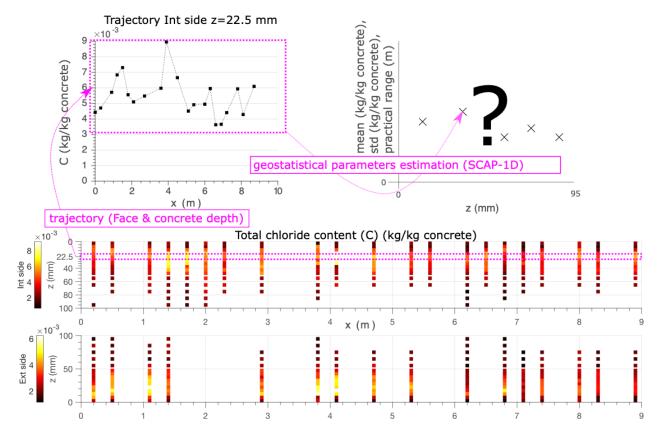


Figure 9 - Total chloride concentration database and illustration of the principle of implementing SCAP-1D on total chloride concentration data

#### 3.3. Average diffusion parameters at 28 years

The 37 total chloride profiles were post-processed for determining associated Fick's second law average diffusion parameters at reference time  $t_r$  of 28 years, noted  $C_{sa}$  and  $D_a$ . The fitting procedure to the peak of the profiles is described by (Othmen et al., 2018) and implemented in Schoefs et al. (2023). Thus, a trajectory for each diffusion parameter per side, located at a height of 30 cm, is built. These include 16 points for the Ext Side and 21 points for the Int Side, with a minimum spacing of 30 cm between measurement points. Table 2 summarizes this information and Figure 10 shows

trajectories. Note that log function is applied to each parameter for (i) simplifying the representation and visual analysis because they are assumed to be lognormally distributed, (ii) using procedures of identification (see section 4.2) available for normally distributed random fields.

*Table 2- Numbering of the trajectories of average scattering parameters (J-beam)* 

trajectory	Side	number of measurement points
$\log (C_{sa})$ 1	Int	21
$\log (D_a)$ 1	Int	21
$\log (C_{sa})$ 1	Ext	16
$\log (D_a) 2$	Ext	16

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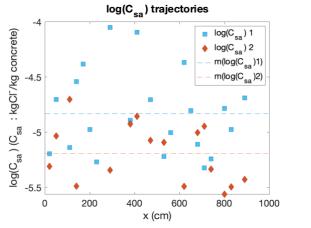
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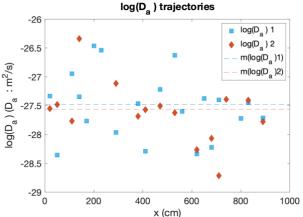
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(a) Trajectories and statistical means of logarithms of the 28-year average surface concentrations of total chlorides - beam J

(b) trajectories and statistical means of the logarithms of the mean diffusivities of total chlorides at 28 years - beam J

Figure 10 - Trajectories and statistical averages of the logarithms of the mean diffusion parameters at 28 years (J-beam)

# 4. Spatial variability estimates of total chloride content in J-beam and physical understanding

#### 4.1. Preamble: trajectories conditioning (kriging, debiasing)

Taking into account the non-usable or missing measurements per specimen and in order to consider the maximum number of points per trajectory, these are first enriched by ordinary kriging of the available data (Baillargeon, 2005, Chapter 4; Chilès & Delfiner, 2012, Chapter 3). The preliminary plot of raw trajectories does not reveal any apparent non-stationarity (Figure 11) and the expectation of their associated RF is not known.

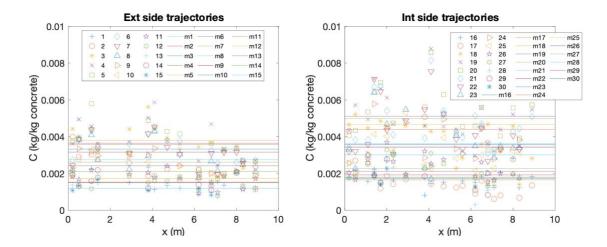


Figure 11 - Raw trajectories and associated statistical means - trajectory number / depth correspondences presented in Table 5

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The kriging is then performed from the identification by weighted Least-Square Estimate (wLSE) of the semi-variograms of the initial trajectories. We consider an exponential covariance model and the existence of an additive measurement error, identified by (Bonnet et al., 2017). Moreover, (Bonnet et al., 2017) also identified a measurement bias  $b = 0.37 \times 10^{-3}$  (kg/kg concrete) associated with the measurement protocol used and with human errors during its implementation. All the measurements are therefore debiased before kriging so that the data considered are now C + b. In order to limit the influence of the kriged data on the estimation of the SV of C, only the trajectories for which their proportion is lower than 25% are considered. A total of 13 trajectories of 16 points can be considered on the Ext side ( $z \in [2.5; 75]$  mm) and 12 trajectories of 21 points on the Int side ( $z \in [2.5; 65]$  mm). The number of kriged data per trajectory is detailed in Table 5 in Appendix, and the selected debiased trajectories are shown in Figure 12. Less than 25% data are kriged until 65-mm depth for both sides. We point out that due to the absence of measurements at the abscissae of the non-exploitable specimens, the trajectory grids are irregular, with 30% of measurements missing for the Int side and 53% for the Ext side. The performances of the SCAP-1D normality tests are therefore affected (Clerc et al., 2019) and particular care must be taken to ensure the consistency of the estimation results.

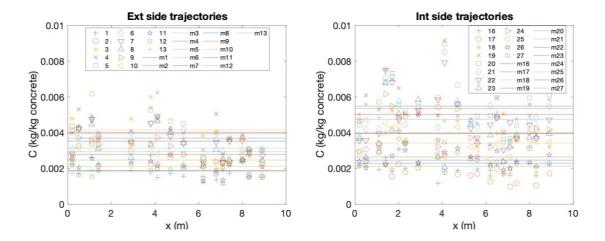


Figure 12 - Selected trajectories and associated statistical means

#### 4.2. Geostatistical pre-study and choice of parameters (Step I):

We recall that the trajectories studied are debiased. We then denote  $\tilde{Z}$  the RF of C such that

$$\tilde{Z} = Z - b$$
 |  $b = -0.37 \times 10^{-3} \text{ kg/kg concrete}$  (4)

Moreover, (Othmen et al., 2018) have previously determined that the marginal law of C is lognormal and (Bonnet et al., 2017) have identified the "measurement error" introduced by the measurement protocol, but for a mortar only. This is distributed according to a generalized extremum law (GeV, (Johnson et al., 1995, p. 3)) with parameters k = 0.16,  $\sigma = 9.3 \times 10^{-5}$  and  $\mu = -8.4 \times 10^{-5}$ . Schoefs et al. (2023) have recently highlighted that the model of error varies according to the chloride content. As a consequence, the error should vary according to each trajectory. For simplicity, it is considered as constant in the paper and its standard deviation is an average of the potential standard deviations (Schoefs et al., 2023).

The best fitting RF model for  $\tilde{Z}$  is then the sum of a log-normal RF Y and a GeV random variable (r.v) W so that, following Equation (1), Z = Y + W.

However, the joint pdf of such a RF is not explicit and SCAP-1D cannot then be simply applied.

The small number of points per trajectory also excludes an estimation by wLSE on semi-variograms

435 (see 2.2.2).

Two modeling compromises are then possible in order to guarantee explicitness:

• <u>Model a</u>: consider a normal marginal distribution of total chloride content and a normal distribution of the error, so that

$$\widetilde{Z} \sim \underbrace{N(\mu(x), s. R(a))}_{G^1} + t. N(0, I)$$
(5)

- where s and t are respectively the empirical variance of the signal and the error and R(a) is the autocorrelation matrix.
- Model b: do not consider the measurement error, whose standard deviation (1.55 × 10<sup>-4</sup> kg/kg concrete according to the model of (Bonnet et al., 2017)) would induce a maximum CoV of 8.3%
   (on trajectory 13; Ext side, z = 75 mm). For this model,

$$\tilde{Z} \sim \exp\left(\underbrace{N(\mu(x), s. R(a))}_{G^2}\right)$$
 (6)

- 444  $G^1$  and LN are then the RF describing the SV of C, with  $G^2$  the source GRF of LN.
- The first approximation of the maximum CoV according to the model of (Bonnet et al., 2017) being significant, we subsequently apply SCAP-1D considering both models so that:
- if the Signal-to-Noise Ratio (SNR) associated with model *a* is low, the error is non-negligible and model *a* is retained;
- if the SNR associated with model *a* is high, the error is considered negligible and model *b* is retained.
- Because of the absence of non-stationarities visible in Figure 12, the trajectories are *a priori* assumed to be stationary, so that  $\mu(x) = \mu$ . We also recall that the autocorrelation function model chosen for  $G^1$  and LN is exponential. According to (Chilès & Delfiner, 2012, p. 106), to guarantee such a model for the autocorrelation function  $\rho_{LN}(a)$  of LN, the autocorrelation function  $\rho_{G^2}$  of  $G^2$  must be written

$$\rho_{G^2}(\cdot) = \ln(\rho_{LN}(\cdot).[e-1] + 1) \tag{7}$$

Still according to (Chilès & Delfiner, 2012, p. 106), the mean  $m_{LN}$  and the variance  $\sigma_{LN}^2$  of LN are then written

$$m_{LN} = \exp\left(\mu + \frac{1}{2}s\right)$$
 (8);  $\sigma_{LN}^2 = m_{LN}^2(\exp(s) - 1)$  (9)

## 4.3. Model a: Parameter estimation, hypothesis testing and analysis (Steps II, III

459 *and IV*)

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460 Preliminary note: Before estimation, the trajectory data, of order  $10^{-3}$ , are multiplied by a factor

461  $k = 10^4$  to facilitate the convergence of the MLE. The estimates of t, s and  $\mu$  are then transformed

462 accordingly before analysis.

Table 6 in Appendix details the results of the SCAP-1D implementation considering model a. Only

the results of the  $\Sigma_{mod}$ -tests are considered due to the small number of points per trajectory.

Analyzing these, the estimates for trajectories 4 and 5 are rejected due to the rejection of the

stationarity hypothesis by the KPSS-test and because the estimated error and practical range are

zero and well below  $2\Delta_x$ , respectively, so that the spatial correlation of the data cannot be described

(see 2.2.5). This is also the case for trajectories 2, 11, 12, and 13 and can be explained by their very

small numbers of points. Similarly, the estimates associated with trajectories 15, 17 and 24 are not

considered because the estimated practical range is zero. Finally, no valid and convergent estimate

can be obtained for trajectory 14. As a first step, we plot for each selected trajectory the estimates

 $\hat{\mu}$  of the mean with their 95% MLE-based confidence intervals in Figure 13 (a)<sup>5</sup>, and the estimates

 $\sqrt{\hat{s}+\hat{t}}$  of the standard deviation in Figure 13 (b), for which the distribution is unknown. We then

note that these are consistent with the statistical mean and standard deviation profiles previously

plotted in Figure 8: the estimated values through kriging (see 4.1) do not create disturbance of these

statistical estimates.

As a second step, we plot in Figure 14 (a) the standard deviation estimates  $\hat{\tau} = \sqrt{\hat{t}}$  and  $\hat{\sigma} = \sqrt{\hat{s}}$  of

the error and the signal respectively as well as the standard deviation  $\tau_{\varepsilon}$  associated with the error

479 model of (Bonnet et al., 2017). Trajectories for which the estimated error is zero are not considered.

<sup>&</sup>lt;sup>5</sup> some values are missing due to the singularity of the Fisher matrix

We then compare in Figure 14 (b) the roots of the experimental SNRs  $\hat{t}/\hat{s}$  and the fixed error SNRs  $\tau_{\varepsilon}^2/\hat{s}$ . Thus, we note that 75% of the roots of the experimental SNRs are greater than 100 and with no specific trend of evolution along. The fixed error SNRs are logically lower than the experimental ones because the error was computed for mortars for which the error of assessment is larger (Schoefs et al., 2023).

In accordance with the previously stated modeling assumptions, we therefore consider the measurement error negligible and retain model b for the SCAP-1D trajectory study.

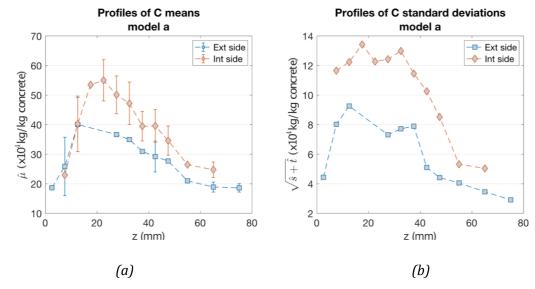
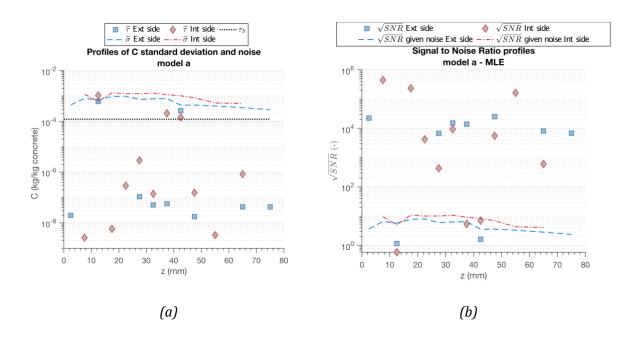


Figure 13 - Profiles of the evolution of the mean and standard deviation of total chloride content according to the depth - model a

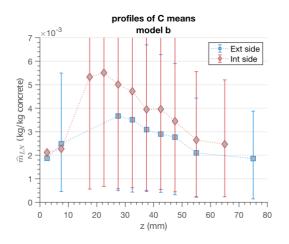


#### 4.4. Model b: Parameter estimation, hypothesis testing and analysis (Steps II, III, IV)

Preliminary note: Since the RF considered in model b is log-normal, the data considered for the SCAP-1D implementation are the logarithms of the debiased data to consider a normal joint pdf. The estimators of  $m_{LN}$ ,  $\sigma_{LN}^2$  and  $r_{95,LN}$  are then calculated from Equations (7) to (9) and from the estimators  $\hat{s}$ ,  $\hat{t}$  and  $\hat{a}$ .

Table 7 in Appendix shows the result of the SCAP-1D implementation considering model b. Only the results of the  $\Sigma_{mod}$ -tests are considered due to the small number of points per trajectory. Analyzing these, the estimates for trajectories 3, 4, 5 and 16 are rejected due to the rejection of the stationarity hypothesis by the KPSS-test. A model of the mean and covariance function that satisfies the hypothesis cannot be found, so no geostatistical parameter estimates are available for these trajectories. 77% of the estimates are then valid on Ext side (10/13) and 92% on Int side (11/12).

The estimates  $\hat{m}_{LN}$  and  $\hat{\sigma}_{LN}$  of the means and standard deviations are plotted with their confidence intervals (CIs) for each trajectory in Figure 15. Note that the estimates are consistent with the statistical mean and standard deviation profiles previously plotted in Figure 8. The estimates  $\hat{r}_{95,LN}$  of the practical ranges, meanwhile, are plotted with their CIs in Figure 16.



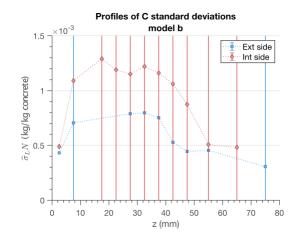


Figure 15 - Profiles of the evolution of the mean and standard deviation of total chloride concentrations according to the depth of embedding - model b

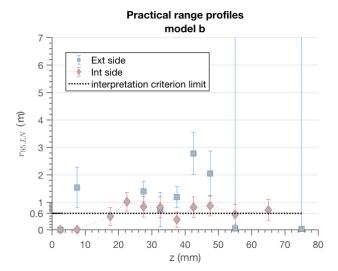


Figure 16 - Practical range profiles of total chloride concentration trajectories as a function of embedment depth - model b

In the case of the trajectories associated with the Ext side, only 8 out of 10 estimates retained by SCAP-1D (i.e., 47% of all estimates) are finally retained after filtering out the outliers. These estimates all meet the interpretability criterion (see 2.2.5) with values between 1 and 3 m for a minimum measurement step of 30 cm. Contrary to the assumptions made at the beginning of the study, however, there is no trend in the evolution of  $\hat{r}_{95,LN}$  as a function of the depth.

In the case of the trajectories associated with the Int side, all the estimates retained by SCAP-1D can be considered after filtering out the outliers (i.e., 92% of all the estimates). In accordance with the hypotheses formulated at the beginning of the study, we observe a zero practical range near the surface, then a quasi-constant value around 60 cm-1 m from 20 mm depth, respecting the interpretability criterion. This tendency reflects a transition from a purely random quantity of interest (q.i) at the surface to a spatially correlated q.i. at depth. We propose following explanation:

• At the surface, many spatio-temporally random material (micro-cracks, ...), physico-chemical (local runoff, ...), and environmental phenomena (biofouling, ...) contribute to the supply and convection of chlorides so that, in coherence with the central limit theorem, the concentration of chlorides at the surface can be considered totally random, at least at the scale of the 30 cm spatial discretization studied;

• at depth, the phenomenon of diffusion and fixation of *C* gradually takes precedence over their distribution within the coating, so that the fixation and diffusion properties of the concrete become predominant compared to the external and surface phenomena. Since the concrete has a certain spatial homogeneity, it is then consistent that *C* is spatially correlated.

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Furthermore, it is noted that the value of  $\hat{r}_{95,LN}$  at the rebar depth is of the same order as the as the conventional spacing of the vibrations during concrete pouring, from 30 to 60 cm (Guiraud, 2018; SETRA, 2005, p. 199). This remark then reinforces the idea that the spatial correlation of C is strongly related to the homogeneity properties of concrete.

#### 4.5. Model b: construction of a unique autocorrelation model of chloride content

The results obtained from the study of the Int side trajectories are much more convincing than those from the study of the Ext side ones. In order to propose a unique autocorrelation model of C for the diffusion region, we then perform an MLE on the set of reduced centered trajectories of the Int side (without the first 10 mm), considering a unique scaling parameter a. Because of the non-stationarity in mean and variance of C according to depth, we consider the trajectories independent of each other (we therefore neglect SV along z-axis). Still considering an exponential covariance model, the estimated practical range is  $\hat{r}_{95} = 0.71$  m, i.e,  $\hat{a} = 0.24$  m. We then state that the Gaussian model of identical practical range has the scale parameter  $\hat{a} = 0.41$ m. The calculation of the ER from the AICc however, does not allow distinguishing between the two models from the data, either by considering each trajectory or from the MLE estimate over all trajectories. Indeed, their values lie between 1 and 1.29. These are reported in Table 8 in Appendix. By considering only the estimation results from the Int side, which are considered more robust due to the larger amount of data per trajectory, we can partially answer the questions raised in the introduction of this study. The SV of chloride content indeed seems to depend on the depth, with a large variability within the convection zone, and much less and more stable in the diffusion zone with a practical range of about 70cm. This value is however more than two to three times lower than

the practical range values of the average diffusion parameters  $C_{sa}$  and  $D_a$  reported in the literature, between 1.76 and 2.64m (Engelund & Sørensen, 1998; Karimi, 2002; O'Connor & Kenshel, 2013). However, we recall that the latter are obtained with less robust methods than SCAP-1D and with even smaller measurement sets. The question of the influence of exposure conditions on the SV of chloride content remains open, however, because of the inconsistency of the practical range estimates on the Ext side. Given the interpretation of the results for the Int side, however, it can be expected that it has an influence only near the surface, for which the chloride contents are already spatially decorrelated. However, this proposal needs to be confirmed by studying an even richer database in order to increase the robustness of the estimates.

## 5. Spatial variability estimation of chloride diffusion parameters of J-beam and physical analysis

#### 5.1. Geostatistical pre-study and choice of procedure parameters (Step I):

to the geostatistical description of the data.

The trajectories of  $\log(C_{sa})$  and  $\log(D_a)$  are shown in Figure 17 to Figure 20, as well as their means and the histograms and experimental semivariogams of centered trajectories. These curves are plotted for each trajectory in three cases: (i) stationary mean (no changepoint); (ii) piecewise stationary mean with one changepoint determined via the PELT algorithm (Killick et al., 2012); (iii) piecewise stationary average at two changepoints determined via the PELT algorithm.

The presence of sills on the experimental semi-variograms allows identifying the *a priori* stationary centered trajectories and thus the most appropriate non-stationarity model for each. The shapes of the histograms and semi-variograms (nugget effect, tangent to the origin, ...) allows identifying models of joint density and a priori autocorrelation function that are adequate

Since the amount of data per trajectory is limited, especially for the Ext side, Step I could only be executed for the trajectories  $\log(C_{sa})$  1 and  $\log(D_a)$  1. The observations made as well as the models considered *a priori* are presented in Table 3.

Table 3- Geostatistical pre-study of the trajectories of the logarithms of the average diffusion parameters – J-beam

		analysis c	of experimental	models considered						
trajectory	chgpt(*)	nugget	oscillations	original tangent	histogram shape	average	field	covariance		
$\log (C_{sa})_1$	0	no	no	insufficient resolution	asymmetrical	cste	GRF	Gauss or expon		
$\log (D_a)_1$	0	no	no	insufficient resolution	symmetrical	cste	GRF	Gauss or expon		
$\log (C_{sa})_2$	-	-	-	-	-	cste	GRF	Gauss or expon		
$\log (D_a)_2$	-	-	-	-	-	cste	GRF	Gauss or expon		
(*): changepo	(*): changepoints necessary for the experimental semi-variogram to have a sill									

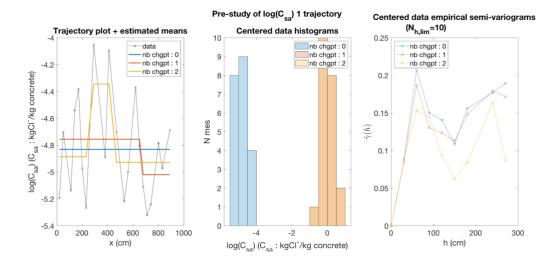


Figure 17- Data and averages of trajectory 1 of log mean surface concentration; histograms of trajectories so centered; experimental semivariogams of trajectories so centered

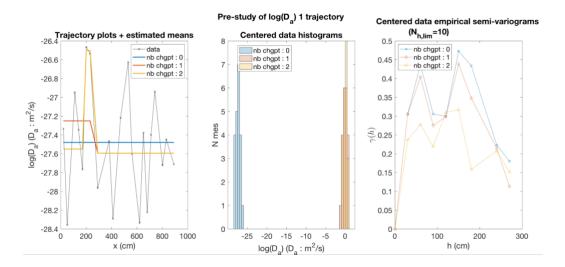


Figure 18- Data and averages of trajectory 1 of logarithm of mean diffusivity; histograms of trajectories so centered; experimental semivariogams of trajectories so centered

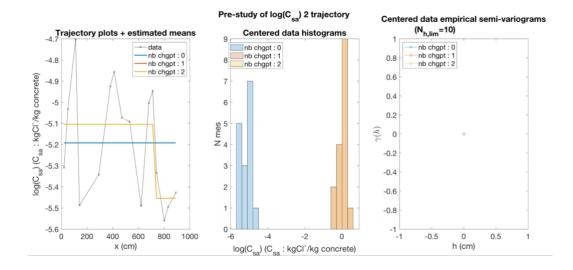


Figure 19- Data and averages of logarithm of mean surface concentration trajectory 2; histograms of trajectories so centered; experimental semivariogams of trajectories so centered

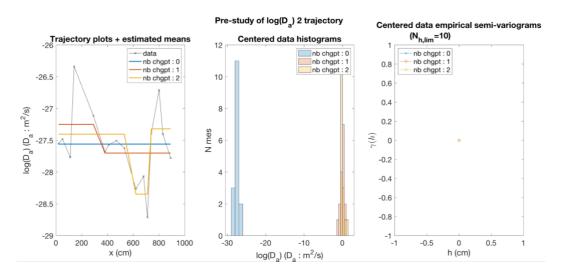


Figure 20- Data and averages of trajectory 2 of logarithm of mean diffusivity; histograms of trajectories so centered; experimental semivariogams of trajectories so centered

#### 5.1.1. Fields models

Although the histogram of the centered trajectory  $\log(C_{sa})$  1 is asymmetric, this may be due to the spatial correlation of the data. The histogram of  $\log(D_a)$  1 is centered. Moreover, we do not notice any nugget effect. This is due to the fact that the measurement noise of the chloride data is negligible (see 4.3). For simplicity and consistency with the results of the literature, a Gaussian Random Field model (GRF) is used to estimate spatial variability. This model is also retained by default for the  $\log(C_{sa})$  2 and  $\log(D_a)$  2 trajectories.

5.1.2. Averaging models: apparent non-stationarity

The trajectories are *a priori* globally stationary since the experimental semi-variograms without any

changepoint in the mean present a sill. A constant mean model is therefore retained.

#### 5.1.3. Covariance function models

Due to the large between measurement points, the spatial resolution is insufficient to visualize the presence of a tangent at the origin of the experimental semi-variograms. We therefore retain both the Gaussian and exponential covariance models without *a priori* preference.

This choice does not allow accounting for the correlation between  $\log(C_{sa})$  and  $\log(D_a)$ , well identified by (Othmen et al., 2018) and which should be modeled via a cross-correlated Random Field (XRF) (Chilès & Delfiner, 2012, p. 332). It is however justified as XRF modelling requires additional parameters (cross-correlation coefficient and cross-scale parameter(s)) which leads to overparametrization and unsuccessful assessment attempts due to the limited amount of data<sup>6</sup>.

#### 5.2. Parameter estimation, hypothesis testing and analysis (Steps II, III and IV)

SCAP-1D steps II and III are applied to the trajectories of  $log(D_a)$  considering the models kept in 5.1. This allows the most appropriate covariance model to be determined from the calculation of Evidence Ratios (ER). These are computed from the AICc. Table 4 shows the results of SCAP-1D execution by trajectory. The models are ranked by increasing value of their ER.

Table 4- Geostatistical parameter estimates of the trajectories of the logarithms of the mean scattering parameters by SCAP-1D

			model			$\Sigma_{exp}$ -tests					$\Sigma_{mo}$	<sub>d</sub> -tests		est	timates	i		CoV		model selection		range	
traj	#	model	cov	chg	reg	$\chi^2$	KS	KPSS	DF	$\chi^2$	KS	KPSS	DF	α1	s	a1 (cm)	α1	s	a1	AICc	ER	r <sub>95</sub> (cm)	r <sub>95</sub> /L
$\log(C_{sa})$ 1	1	GRF	expon	0	0	0	0	0	1	0	0	0	1	-4,82	0,13	17	0,02	0,31	0,81	-14,08	2,13	50	0,05
log(c <sub>sa</sub> ) i	2	GRF	gauss	0	0	0	0	0	1	0	0	0	1	-4,81	0,15	37	0,02	0,39	0,33	-15,59	1,00	63	0,07
$\log(C_{sa})$ 2	1	GRF	expon	0	0	0	0	0	1	0	0	0	1	-5,20	0,07	19	0,01	0,36	1,20	-19,22	1,00	57	0,06
log(c <sub>sa</sub> ) 2	2	GRF	gauss	0	0	0	0	0	1	0	0	0	1	-5,20	0,07	20	0,01	0,36	0,80	-19,11	1,06	35	0,04
$log(D_a)$ 1	1	GRF	expon	0	0	0	0	0	1	0	0	0	1	-27,48	0,31	10	0,00	0,31	1,65	3,78	1,01	29	0,03
$\log(D_a)$ 1	2	GRF	gauss	0	0	0	0	0	1	0	0	0	1	-27,48	0,31	18	0,00	0,31	0,77	3,76	1,00	32	0,03
$log(D_a)$ 2	1	GRF	expon	0	0	0	0	0	1	0	0	0	1	-27,56	0,29	13	0,01	0,35	1,40	4,19	1,00	40	0,04
$\log(D_a)$ 2	2	GRF	gauss	0	0	0	0	0	1	0	0	0	1	-27,56	0,29	19	0,01	0,35	0,72	4,21	1,01	33	0,04

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<sup>&</sup>lt;sup>6</sup> MLE on raw data and MLE on data enriched via Metropolis-Hastings sampling did not give satisfactory results

First no estimate is rejected by the hypothesis tests. The estimates computed from the exponential and Gaussian models are then consistent for each of the trajectories except for the trajectory  $\log(C_{sq})$ 2 (relative errors of respectively 21%, 63%, 9% and 21% between the practical ranges of the Gaussian model and those of the exponential model). Furthermore, the exponential and Gaussian covariance models cannot be distinguished from the ERs (maximum difference observed for the  $\log(C_{sq})$  1 trajectory with a ratio of 1 for the Gaussian model and a ratio of 2.13 for the exponential model). However, the Gaussian model leads to smaller coefficients of variation of the geostatistical parameters (up to a factor of 2.5 for the scale parameter of trajectory  $\log(C_{sa})$  1). Second, the majority of the practical range estimates do not meet the interpretability criterion (see 2.2.5). All of the estimates of  $r_{95}$  made on the  $\log(D_a)$  trajectories as well as the estimate made on the  $\log(C_{sq})$  2 trajectories are indeed of the order of 30 cm, which is the minimum spacing between measurements. Therefore, only the estimates made on the  $log(C_{sa})$  1 trajectory are retained. The estimated practical range is indeed 50 to 63 cm depending on the chosen covariance model, exponential or Gaussian (i.e., a ratio  $r_{95}/L$  of 0.05 to 0.07). The latter can then be preferred a priori because of its minimal ER even if exponential model is close. Thus, due to the too large spacing between measurement points compared to the hidden value of their practical range, the question of whether the SV of the scattering parameters is a function of the exposure conditions cannot be answered here. Furthermore, the interpretable estimates of practical ranges differ significantly from the 1 to 2 m values estimated by (O'Connor & Kenshel, 2013) on a bridge exposed to chloride ion penetration. Such a difference can be explained by the fact that these authors significantly enriched their initial kriging database (number of points multiplied by 7) before producing their estimates, which may have influenced them significantly.

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#### 6. Conclusion

- In this paper, a new database is proposed for analysis of the spatial variability of chloride ingress into concrete. A total of 460 measurements was available, belonging to 2 exposures zones: 12 trajectories of 21 points for each profile on the Internal side (sheltered from wetting-drying cycles) and 13 trajectories of 16 points on the External side. This data base was uses to analyze the spatial variability of (i) chloride content, (ii) parameters of a model of chloride diffusion. The analysis is performed on the basis of a complete framework (SCAP-1D) which raises, rigorously for the first time, questions of stationarity, marginal distribution and auto-correlation.
- The application to the total chloride content data showed that:
- the measurement error on total chloride content is negligible for the estimation of their spatial variability;
  - near the surface, spatio-temporally random physico-chemical and environmental phenomena contribute to the diffusion and convection of chlorides so that, in coherence with the central limit theorem, total chloride content at the surface can be considered totally random;
    - in depth (from about 2 cm), total chlorides diffusion and fixation gradually take precedence over their distribution within the coating, so that fixation and diffusion properties of concrete become predominant compared to the external and surface phenomena. Total chloride content is then spatially correlated with an estimated general practical range of 71 cm (scale parameter of 24 cm) which is of the same order as the conventional vibration spacing during concrete pouring (30-60 cm (Guiraud, 2018; SETRA, 2005, p. 199)). This supports the fact that total chloride content spatial correlation in depth is strongly related to the homogeneity properties of the concrete resulting from pouring.
    - The application to the identification of diffusion parameters showed that:
- The application to the chloride diffusion parameters data was presented in section 5. It allowed estimation of geostatistical parameters only for the logarithm of the average surface chlorides concentration. This is due to a limited number of points per trajectory (from 16 to 21) and a too large

spacing between measurement points (30 cm). The estimated practical range value is nevertheless consistent with that of the total chloride content in depth since it is between 50 and 63 cm. Implementation of SCAP-1D on these two datasets allowed to note that when the number of points per trajectory is limited but not small ( $\geq$  20), it is more efficient to first consider the results of statistical tests performed on the standardized trajectory from the empirical covariance matrix, and then in a second step those performed on the standardized trajectory from the modeled covariance matrix. This allows to first assess the validity of the mean model, then those of the marginal density model and the estimates of the geostatistical parameters of the source Random Field. On the contrary, when the number of points per trajectory is small (< 20), it is preferable to consider only the results of the statistical tests performed on the standardized trajectory from the modeled covariance matrix, in order to guarantee its positivity of the latter.

### 7. Appendix – Detailed tables of estimation process data

### Table 5- Definition of trajectories and proportion of kriged data per trajectory – Ext side and Int side

	kriged data -	Ex side	!		kriged data	Int side	
trajectory	depth (mm)	number	percentage	trajectory	depth (mm)	number	percentage
1	2.5	0	0%	16	2.5	0	0%
2	7.5	0	0%	17	7.5	1	5%
3	12.5	0	0%	18	12.5	1	5%
4	17.5	0	0%	19	17.5	1	5%
5	22.5	0	0%	20	22.5	2	10%
6	27.5	0	0%	21	27.5	1	5%
7	32.5	0	0%	22	32.5	2	10%
8	37.5	2	13%	23	37.5	2	10%
9	42.5	3	19%	24	42.5	4	19%
10	47.5	4	25%	25	47.5	3	14%
11	55	1	6%	26	55	4	19%
12	65	1	6%	27	65	4	19%
13	75	3	19%	28	75	9	43%
14	85	9	56%	29	85	15	71%
15	95	10	63%	30	95	17	81%
	Total	33	14%		Total	66	21%

Table 6 - Estimation of geostatistical parameters of total chloride concentration trajectories by SCAP-1D - model a

	trajectory model							Σexp-tests					Σmoc	I-tests		estin	nates (da	ta multipli	ied by k=	=10e4)	Coefficient of Variation (CoV)				
#	Side	z (mm)	Npts	model	cov	reg	chg	chi2	KS	KPSS	DF	chi2	KS	KPSS	DF	μ	s	t	а	r95	CoV μ	CoV s	CoV t	CoV a	
1	Ext	2,5	16	GW	expon	0	0	0	0	0	1	0	0	0	1	18,66	19,63	0,00	0,01	0,03	NaN	NaN	NaN	NaN	
2	Ext	7,5	16	GW	expon	0	0	0	0	0	1	0	0	0	1	25,86	64,36	0,00	0,00	0,00	2,0E-01	1,2E+00	5,7E <del>-</del> 01	1,7E+00	
3	Ext	12,5	16	GW	expon	0	0	1	0	1	0	0	0	0	1	40,04	49,27	36,47	2,57	6,28	1,2E-01	8,9E-01	5,3E-01	1,3E+00	
4	Ext	17,5	16	GW	expon	0	0	0	0	0	1	0	0	1	1	41,59	91,14	0,00	0,01	0,01	5,7E <b>-</b> 02	7,6E+02	1,8E+02	3,8E+05	
5	Ext	22,5	16	GW	expon	0	0	0	0	1	0	0	0	1	1	39,44	96,31	0,00	0,01	0,01	NaN	NaN	NaN	NaN	
6	Ext	27,5	16	GW	expon	0	0	1	0	0	1	0	0	0	1	36,65	53,47	0,00	0,52	1,55	NaN	NaN	NaN	NaN	
7	Ext	32,5	16	GW	expon	0	0	1	0	0	1	0	0	0	1	34,95	59,57	0,00	0,32	0,96	NaN	NaN	NaN	NaN	
8	Ext	37,5	16	GW	expon	0	0	0	0	0	1	0	0	0	1	30,95	62,31	0,00	0,37	1,12	NaN	NaN	NaN	NaN	
9	Ext	42,5	16	GW	expon	0	0	0	0	0	1	0	0	0	1	29,20	18,89	7,15	2,17	5,80	9,1E-02	7,5E <b>-</b> 01	7,5E-01	1,1E+00	
10	Ext	47,5	16	GW	expon	0	0	0	0	0	1	0	0	0	1	27,69	19,49	0,00	0,71	2,12	NaN	NaN	NaN	NaN	
11	Ext	55	16	GW	expon	0	0	0	0	0	1	0	0	0	1	20,98	16,46	0,00	0,01	0,02	NaN	NaN	NaN	NaN	
12	Ext	65	16	GW	expon	0	0	0	1	1	0	0	0	0	1	18,89	11,95	0,00	0,01	0,03	4,6E <b>-</b> 02	1,5E+02	9,6E+09	5,6E+05	
13	Ext	75	16	GW	expon	0	0	0	0	0	1	0	0	0	1	18,61	8,47	0,00	0,01	0,03	3,9E-02	1,9E+02	9,0E+09	1,5E+05	
14	Int	2,5	21	GW	expon	0	0	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	NaN	
15	Int	7,5	21	GW	expon	0	0	0	0	0	1	0	0	0	1	22,94	135,81	0,00	0,00	0,00	NaN	NaN	NaN	NaN	
16	Int	12,5	21	GW	expon	0	0	1	1	0	1	0	0	0	1	40,27	38,35	111,41	3,19	5,21	1,2E-01	1,3E+00	3,6E-01	1,5E+00	
17	Int	17,5	21	GW	expon	0	0	0	0	0	1	0	0	0	1	53,50	180,15	0,00	0,00	0,00	NaN	NaN	NaN	NaN	
18	Int	22,5	21	GW	expon	0	0	0	0	0	1	0	0	0	1	55,01	150,55	0,00	0,34	1,01	6,5E <b>-</b> 02	1,1E+00	1,8E+07	1,0E+00	
19	Int	27,5	21	GW	expon	0	0	0	0	0	1	0	0	0	1	50,09	154,45	0,00	0,24	0,72	6,5E <b>-</b> 02	1,8E+00	3,2E+05	1,1E+00	
20	Int	32,5	21	GW	expon	0	0	0	0	0	1	0	0	0	1	47,18	168,62	0,00	0,31	0,93	7,8E <b>-</b> 02	9,9E <b>-</b> 01	8,2E+07	8,8E-01	
21	Int	37,5	21	GW	expon	0	0	0	0	0	1	0	0	0	1	39,46	126,85	4,31	0,09	0,26	6,4E-02	2,6E+01	7,8E+02	7,8E+00	
22	Int	42,5	21	GW	expon	0	0	0	0	- 1	0	0	0	0	1	39,57	103,23	2,06	0,29	0,86	7,2E <b>-</b> 02	1,2E+00	5,9E+01	1,1E+00	
23	Int	47,5	21	GW	expon	0	0	1	0	0	1	0	0	0	1	34,59	72,53	0,00	0,35	1,06	7,3E <b>-</b> 02	9,4E <b>-</b> 01	2,7E+07	8,5E <del>-</del> 01	
24	Int	55	21	GW	expon	0	0	0	0	0	1	0	0	0	1	26,46	28,31	0,00	0,00	0,00	NaN	NaN	NaN	NaN	
25	Int	65	21	GW	expon	0	0	0	0	0	1	0	0	0	- 1	24,73	25,35	0,00	0,25	0,74	5,4E-02	1,9E+00	6,9E+05	1,2E+00	

GW : Gaussian + additive white noise; reg : regression degree of mean model; chg : maximum number of changepoints of mean model

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	trajectory model							o-tests		Σmo	d-tests		estimates	s (logarithm	of data)	Coefficients of Variation (CoV)					
#	Face	z (mm)	Npts	modèle	cov	reg	chg	chi2	KS	KPSS	DF	chi2	KS	KPSS	DF	μ	s	а	CoV μ	CoV s	CoV a
1	Ext	2,5	16	G	expon	0	0	0	1	0	1	0	0	0	1	6,31	0,05	0,00	NaN	NaN	NaN
2	Ext	7,5	16	G	expon	0	0	0	1	1	1	0	0	0	1	6,04	0,08	0,52	1,6E-02	4,2E-01	7,5E-01
3	Ext	12,5	16	G	expon	0	0	0	0	0	1	0	0	1	1	5,54	0,06	0,26	1,3E-02	3,7E-01	8,4E-01
4	Ext	17,5	16	G	expon	0	0	0	0	0	. 1	0	0	1	1	5,51	0,05	0,00	1,0E-02	3,5E-01	1,8E+06
5	Ext	22,5	16	G	expon	0	0	0	0	1	1	0	0	1	1	5,57	0,07	0,13	1,2E-02	3,5E-01	1,6E+00
6	Ext	27,5	16	G	expon	0	0	0	0	1	1	0	0	0	1	5,63	0,05	0,47	1,3E-02	4,0E-01	6,4E-01
7	Ext	32,5	16	G	expon	0	0	0	1	1	1	0	0	0	1	5,68	0,05	0,25	1,1E-02	3,7E-01	9,4E-01
8	Ext	37,5	16	G	expon	0	0	0	0	0	1	0	0	0	1	5,81	0,06	0,40	1,4E-02	3,9E-01	6,7E-01
9	Ext	42,5	16	G	expon	0	0	0	1	1	0	0	0	0	1	5,86	0,03	0,93	1,3E-02	4,8E-01	6,5E-01
10	Ext	47,5	16	G	expon	0	0	0	1	1	1	0	0	0	1	5,90	0,03	0,69	1,1E-02	4,6E-01	7,2E-01
11	Ext	55	16	G	expon	0	0	0	0	0	1	0	0	0	1	6,19	0,05	0,02	8,7E-03	3,5E-01	4,8E+02
12	Ext	65	16	G	expon	0	0	0	0	0	1	0	0	0	1	6,29	0,04	0,01	NaN	NaN	NaN
13	Ext	75	16	G	expon	0	0	0	0	0	1	0	0	0	1	6,30	0,03	0,01	6,5E-03	3,5E-01	2,0E+05
14	Int	2,5	21	G	expon	0	0	0	0	0	1	0	0	0	1	6,18	0,05	0,00	NaN	NaN	NaN
15	Int	7,5	21	G	expon	0	0	0	0	0	1	0	0	0	1	6,19	0,21	0,00	NaN	NaN	NaN
16	Int	12,5	21	G	expon	0	0	0	0	0	1	0	0	1	1	5,58	0,14	0,01	1,5E-02	3,1E-01	1,5E+05
17	Int	17,5	21	G	expon	0	0	0	0	0	1	0	0	0	1	5,26	0,06	0,16	1,1E-02	3,1E-01	7,8E-01
18	Int	22,5	21	G	expon	0	0	0	- 1	0	1	0	0	0	1	5,23	0,05	0,34	1,2E-02	3,4E-01	5,3E-01
19	Int	27,5	21	G	expon	0	0	0	1	0	1	0	0	0	1	5,32	0,05	0,28	1,2E-02	3,3E-01	5,8E-01
20	Int	32,5	21	G	expon	0	0	0	1	0	1	0	0	0	1	5,39	0,06	0,28	1,3E-02	3,3E-01	6,0E-01
21	Int	37,5	21	G	expon	0	0	0	1	0	1	0	0	0	1	5,58	0,08	0,13	1,2E-02	3,1E-01	1,0E+00
22	Int	42,5	21	G	expon	0	0	0	0	0	1	0	0	0	1	5,57	0,07	0,28	1,3E-02	3,3E-01	5,7E-01
23	Int	47,5	21	G	expon	0	0	0	1	0	1	0	0	0	1	5,70	0,06	0,30	1,2E-02	3,3E-01	5,7E-01
24	Int	55	21	G	expon	0	0	0	0	0	1	0	0	0	1	5,95	0,04	0,19	7,9E-03	3,1E-01	6,4E-01
25	Int	65	21	G	expon	0	0	0	0	0	1	0	0	0	1	6,02	0,04	0,24	8,4E-03	3,2E-01	5,9E-01

G : Gaussian; reg : regression degree of mean model; chg : maximum number of changepoints of mean model

Table 8 - Computation of the evidence ratios associated with the exponential and Gaussian covariance models for model b using the corrected Akaike criterion

	tr	ajectory		n	nodel		Corrected Al	kaike criteria	Evidence Ratios		
#	Face	z (mm)	Npts	model	reg	chg	cov gauss	cov expon	cov gauss	cov expon	
17	Int	17.5	21	G	0	0	-3.21E+01	-3.22E+01	1.04	1.00	
18	Int	22.5	21	G	0	0	-3.89E+01	-3.89E+01	1.00	1.05	
19	Int	27.5	21	G	0	0	-3.59E+01	-3.55E+01	1.00	1.21	
20	Int	32.5	21	G	0	0	-3.05E+01	-3.07E+01	1.11	1.00	
21	Int	37.5	21	G	0	0	-2.39E+01	-2.39E+01	1.00	1.01	
22	Int	42.5	21	G	0	0	-2.89E+01	-2.94E+01	1.29	1.00	
23	Int	47.5	21	G	0	0	-3.14E+01	-3.18E+01	1.18	1.00	
24	Int	55	21	G	0	0	-4.21E+01	-4.18E+01	1.00	1.14	
25	Int	65	21	G	0	0	-4.22E+01	-4.18E+01	1.00	1.26	

G: gaussian

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